

ADDENDUM

Project: Grand Lake St. Mary's State Park, Park Office Renovation
Project No.: DNR-250013
Date: October 21, 2025
Addendum No.: 1

This addendum must be receipted for on the proposal form.

TO ALL BIDDERS:

This addendum supplements and amends the original drawings and project manual and shall be taken into account in preparing proposals and shall become a part of the contract document.

NOTE: Addenda are mailed only to Prime Contractors and Plan Rooms who obtained plans and specifications from the agency responsible for distribution of said plans and specifications. Each Prime Contractor is responsible for notifying subcontractors of any addenda issued.

Item 1 – Pre-Bid Meeting

- See attached Pre-Bid Sign-In Sheet and notes from the voluntary pre-bid held on October 21, 2025.

Item 2 – Utility Clarification

- When reusing existing underground utilities contractor shall document all existing conditions prior to starting construction such as but not limited to electrical, storm drains and culverts, sewer, water supply, landscaping, pavement, side-walks, and standing water areas.

Item 3 – Specification Section 00 31 32 Geotechnical Investigations

- Add the entire Specification Section 00 31 32 Geotechnical Investigations to the specifications.

Item 4 – Specification Section 00 73 00 Supplementary Conditions (General Contracting)

- See attached HB 96 changes that are also to be included

Item 5 – Specification Section 09 91 00 Paints and Stains

- Under 2.3.B.1 and 2, provide Sunnyside M-1 Advanced Insecticide Additive within coating. Additive product mixes one per gallon of paint/stain and protects against most common types of insects. Follow paint and additive manufacturers' recommendations.

Item 6 – Specification Section 10 51 13 Lockers (Revised)

- See revised specification section attached with a revised locker quantity under 2.2A and the addition of a modified ADA accessible law enforcement locker under 2.2B.

Item 7 – Specification Section 26 36 00 Transfer Switches (Revised)

- See revised specification section for generator inlet and connection cord.

Item 8 – A500.1 (Revised)

- Before final floor finish is installed in drained areas of restrooms, floors shall be water tested / flooded to insure no puddling and positive drainage occurs before final finish coat is applied.

Item 9 – A900.4 (Revised)

- One of the lockers in the office 116 interior elevation K200.1/A900.4 was changed to an ADA locker.

Item 10 – ESP1 & ESP2 (Revised)

- Adjust “campfire connect”, Wi-Fi equipment to align with architecture location within cupola.

Item 11 – E100.1 (Revised)

- (1) Additional battery option has been added to light fixture “B” in the corridor. See revised sheet E100.1. Total of 4 emergency lights in the corridor.

Item 12 – E200.1 (Revised)

- Eliminate the generator connection to match the existing connection. Instead provide a new 50A generator inlet connection and a new generator connection cord.
- Battery option has been added to light fixture “F”. See revised sheet E100.1.
- Circuit A-2 serves a dedicated microwave outlet inside a kitchen cupboard. Outlet has been revised to GFI. Circuit A-12, MDP-13, and various other circuits have GFI protection at the breaker. This is indicated in the panel schedule with a “G” next to the amperage rating of the breaker. The outlet next to the electrical panels has been revised to a GFI outlet. See revised sheet E200.1

Item 13 – E300.1 & E300.2 (Revised)

- Modify data locations per CD submission comments

Item 14 - Question & Answers

- Question: *Can you please advise if signage and other specifications will be provided? Drawing A500.1 says “Note: for additional signage info see specs”.*
 - Answer: Please download all bid documents from Bid Express which include the requested items.
- Question: *I am looking at your project for the GLSM Park office. Do you have specifications on the stone, sills, and split face CMU? If not, I would like to provide samples and answer any questions you may have. Please let me know the best way to proceed.*
 - Answer: Please download all bid documents from Bid Express which include the requested items.
- Question: *Is this an OAKS CI project?*
 - Answer: Yes, review all bid documents from Bid Express with reference to use of the State’s web-based software for the project.

Attachments:

- Pre-Bid Sign-In Sheet and notes
- New Specification Section 00 31 32 Geotechnical Investigations
- Changed Specification Section 00 73 00 Supplementary Conditions
- Revised Specification Section 10 51 13 Lockers
- Revised Specification Section 26 36 00 Transfer Switches
- Revised Drawing A500.1
- Revised Drawing A900.4
- Revised Drawing ESP1
- Revised Drawing ESP2
- Revised Drawing E100.1
- Revised Drawing E200.1
- Revised Drawing E300.1
- Revised Drawing E300.2

END OF ADDENDUM

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Pre-Bid Meeting Template



Project: Park Office Renovation
Facility: Grand Lake St. Marys State Park
Date: 10/21/25
Location: Virtual (Zoom Meeting)
Bid Package: General Trades (Lead)

1. INTRODUCTION

- a. All attendees shall sign the attendance sheet.
- b. Any questions about the plans and specifications should be directed to AE Project Manager Michael Rowe, Phone Number 419-893-9021.

2. PROJECT OVERVIEW

- a. Grand Lake St. Marys Park Office and Camp Store project involve the construction of a new combined park office and camp store. Major items of work include: new building construction, associated site improvements (parking and sidewalks), utility improvements (electric, water and sanitary), demolition.

3. DELIVERY METHOD

- a. General Trades (Lead)

4. NOTICE TO BIDDERS

- a. Bid Opening: November 6, 2025 at 2:00 pm EST/EDT.
- b. Bids will be posted by 5:00 pm the day of opening on Bid Express. Bidders are not required to attend the official Bid Opening.
- c. Plans and Specifications are available for download from Bid Express.
- d. Review contracts and estimated costs:
 - 1) Base Bid - \$2,287,759.00 (includes Lump Sum items)
 - 2) Alternate 1 - \$18,070.00 (add from Base Bid)
 - Ceramic Tile
 - 3) Alternate 2 – \$221,107.00 (add from Base Bid)
 - Standing Seam Metal Roof Instead of Shingle Roof
 - 4) Alternate 3 – \$30,888.00 (add from Base Bid)
 - Additional Casework and Casework Upgrades
 - 5) Alternate 4 – \$103,039.00 (add from Base Bid)
 - Pedestrian Lighting and Additional Lighting

5. SUBMITTING A BID

- a. The Contractor is responsible for delivering the bid by the time specified in Bid Express, <https://bidexpress.com> on the electronic Bid Form in Bid Express.
- b. Note: The Bidder will need to allow several days to receive electronic signature from Bid Express if they have not used Bid Express prior.

6. BID SCHEDULE

- a. Review format of bid schedule and how to properly complete
- b. Allowance Items, Section 01 21 00, Summary of Work
 - i. A-1 (Utility Fees \$30,000.00)
 - ii. A-2 (FFE \$60,000.00)
- c. Alternate Bid Items, Section 01 23 00, Summary of Work
- d. Unit Price Items (N/A)
- e. Addenda - Each Bidder shall bear the responsibility to satisfy himself/herself that the bid is responsive to all Addenda issued. The contractor shall acknowledge any addendum in the

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Pre-Bid Meeting Template



appropriate location on Bid Express. Failure to receive or acknowledge any Addenda shall not release the Bidder from all obligations contained in such Addenda.

- f. The contractor shall not modify anything on the bid schedule as this may be cause for rejection of bid.

7. CONSTRUCTION SCHEDULE

- a. Anticipated NTP Date is January 15, 2026
- b. Scope Review
- c. Milestone Dates, Section 01 32 16:
 - January 15, 2026 – Anticipated Notice to Proceed; construction duration begins
 - March 31, 2026 – Complete seasonal tree clearing within bat habitat areas
 - April 1, 2026 (TBD) – Groundbreaking Ceremony
 - October 1, 2026 (TBD) – Topping out Ceremony
 - April 2, 2027 – Construction duration for new building to be completed
 - April 5, 2027 thru April 30, 2027 – Owner move-in / transfer and start-up activities to be completed within new building
 - May 3, 2027 thru May 28, 2027 – Existing building demolition / site restoration
 - May 31, 2027 – Substantial completion (including punch list completion)
 - June 1, 2027 – Ribbon cutting
- d. Anticipated Date of Substantial Completion of Work on May 31, 2027
- e. Final Closeout Docs Submitted on June 30, 2027

8. BIDDING AND CONTRACTING REQUIREMENTS

- a. Instructions to Bidders - Refer to Documents 00 21 13 & 00 22 00 of Project Manual
 - 1) Proposal Form, Bid Guaranty, Insurance, Bonding, etc. - Questions should be directed to Anthony Hall, ODNR Division of Engineering at (614) 265-6948 or email anthony.hall@dnr.ohio.gov.
 - 2) Requests for Information (RFI's) must be submitted 7 Days before bid opening.
 - 3) All questions asked during the meeting are to be submitted formally in a RFI to ensure inclusion in future Addenda.
 - 4) Substitutions - All bids will be evaluated based on the standard referenced in the documents. Substitutions Requests must be submitted 10 days before bid opening.
 - i. Proposed substitutions must include side-by-side comparison with specs of basis of design to be considered by AE.
 - ii. The name and complete description of the proposed Substitution, including Drawings, performance and test data, and other information necessary for a complete evaluation.
 - iii. Contractor to submit a statement setting forth any changes that the Proposed Substitution will require in the Contract Documents or the Project.
 - iv. If the A/E approves the Proposed Substitution, the A/E shall issue an Addendum.
 - 5) Bid Package Checklist - The following items are the minimum that are to be included in the proposal package.
 - i. Form of Bid
 - ii. Corporation Signing Authority form uploaded
 - iii. Request for Taxpayer ID Number uploaded
 - iv. Bid Security Form (Bid Bond or Cashier's Check) – Original to DNR within three days of bid
 - v. Cert. of Compliance showing Surety is licensed to do business in Ohio

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- vi. Power of Attorney for Surety Form uploaded
- vii. Drug Free Workplace Compliance Certificates
- viii. Bid Schedule
- ix. Proposed EDGE Certified Business/ EDGE Program Commitment
- x. Bidder Qualifications
- xi. Bidder Affirmation and Disclosure
- xii. Bidder Certification Box Checked

9. REQUIRED CONDITIONS PRECEDENT

- a. EDGE
- b. DFSWP
- c. EEO
- d. Insurance Certificate
- e. Insurance Policies
- f. Original of Bond
- g. W-9
- h. Builder's Risk Insurance Certificate and policy
- i. Workers Compensation Certificate
- j. Standard Affirmation and Disclosure (Executive Order 2022-02D)
- k. Certificate of Good Standing (Foreign Corporation Only)
- l. Power of Atty. Designating State of Ohio as Bidders Agent for accepting service of summons (Non-Resident of State Individuals Only)
- m. Contractor License (if Required)
- n. Signed Contracts
- o. WAGES AND HOURS
 - 1) This is a state of Ohio Prevailing Wage Project and any questions should be directed to Prevailing Wage Coordinator (Haley Ingram) ODNR Division of Engineering, (614) 809-7568 or email haley.Ingram@dnr.ohio.gov
- p. EDGE AFFIDAVIT - SECTION 00 45 39
 - 1) Refer to section to assist contractor in submitting required certificate with bid.
 - 2) This project has a 5% EDGE Participation goal minimum.
 - 3) If bidder does not commit to the published EDGE goal the Bidder will need to seek a waiver and demonstrate that good faith efforts were exhausted to reach that goal. A waiver is extremely difficult to obtain. It is best to select Option A on Bid Express to meet or exceed the goal.
 - 1. EDGE Business Search website -
<https://development.force.com/ODSA/s/mbddcertsearch>

10. SAFETY

- a. Contractor shall design and implement its own safety program and submit to Consultant for review.
- b. Hard Hats, Safety Glasses, & Proper Attire
- c. Housekeeping / Site Clean-up
- d. Dig Permit, Welding and Cutting Permit
- e. Fire Prevention
- f. Lead & Asbestos Awareness Training (on renovation projects as required)

11. SCOPE OF WORK

- a. Contract Documents
 - 1) Discuss the items of work in detail and review plans and specifications as necessary.

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12. WORK OR SERVICES BY OTHERS

- a. Construction Testing to be performed by Verdantas as a responsibility by Buehrer Group Architecture & Engineering, Inc. who will coordinate.
- b. Utilities to be supplied by owner for Water, Sewer, Electric, and Natural Gas.
 - 1) Utilities to be paid for by ODNR
- c. FFE to be supplied by and installed by owner
- d. Abatement Work to be performed by contractor
- e. Air Monitoring to be performed by contractor
- f. Commissioning to be performed by contractor

13. PERMITS

- a. Review permits and responsibility.
 - i. Plan review is in process thru Department of Commerce

14. PROJECT RESTRICTIONS/SITE USAGE (Refer to Division 1 of General Requirements)

- a. Occupied Building / Shared Site
- b. Contractor shall prepare and submit a Site Utilization Plan
- c. Access to Site or Building / Haul Roads
- d. Site Security / Fencing
- e. Job Storage
- f. Staging/Laydown Areas
- g. Parking
- h. Work Hours
- i. Temporary Facilities
- j. Field Office for Construction Administration
- k. Protection of Existing Utilities
- l. Noise and Vibrations
- m. Dust & Fume Control
- n. Demolition Waste
- o. Recycling/Waste Plan
- p. Use of Drones requires aviation liability insurance coverage and FCC operator's license.

15. COMMON BIDDER PITFALLS (that will delay contracting process or possibly cause rejection)

- a. Not submitting original bond within 3 days of bid date.
- b. EDGE Option A, B or C is not selected on the EDGE Commitment page in the Bid Form.
- c. Bidder fails to enter a \$ amount, "no change", or \$0 for All Alternates in the Bid Form
- d. Company name on Bid Form or Bond does not match the S.O.S. Business Filings.
- e. Expired Certificates (EEO, Workers Comp., Drug Free)
- f. Bidder is not responsive to the time sensitive nature of the Bidder's Qualifications Request or Notice of Intent to Award submittal requirements.

16. VISIT PROJECT SITE

- a. Bidders are welcome to walk the site and areas of work in advance of submitting bids.

1. **PART 1 - GENERAL**

- 1.1 This report, reprinted herein in its entirety, was compiled by and is the sole responsibility of CT Consultants, Inc. and is provided for informational purposes only.

◆ END OF SECTION ◆



Geotechnical Subsurface for Proposed GLSM State Park Office Renovation 834 Edgewater Drive, St Marys, Ohio

Prepared for:

Michael Rowe
Buehrer Group
314 Conant Street
Maumee, OH 43537
419.893.9021

Prepared by:

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Toledo, OH 43604
419.241.4440

Verdantas Project No: 30131

May 2025



Mr. Michael Rowe
Buehrer Group
314 Conant Street
Maumee, OH 43537
419.893.9021

**Geotechnical Subsurface Investigation for
Proposed GLSM State Park Office Renovation
834 Edgewater Drive, St Marys, Ohio**

Dear Mr. Rowe:

The following geotechnical subsurface investigation report has been prepared for the referenced site by Verdantas. This study was conducted in accordance with Verdantas' Proposal No. 30131 dated October 22, 2024. Authorization to proceed was granted via a signed proposal dated January 7, 2025.

This report contains the results of our study, our engineering interpretation of the results with respect to the project characteristics, and design and construction recommendations for the proposed foundations and pavements.

Soil samples collected during this investigation will be stored at our laboratory for 90 days from the date of this report. The samples will be discarded after this time unless you request that they be saved or delivered to you.

Should you have any questions regarding this report or require additional information, please contact our office.

Sincerely,

Verdantas LLC



Negoslav Tosanovic, P.E.
Geotechnical Project Manager



Curtis Roupe, P.E.
Associate Vice President
Group Leader, Geotechnical Engineering

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PLATES

Plate 1.0 Site Location Map

Plate 2.0 Test Boring Location Plan

FIGURES

- A. Logs of Test Borings
- B. Legend Key
- C. Tabulation of Test Data
- D. Laboratory Test Results

1. INTRODUCTION

This geotechnical subsurface investigation report has been prepared for the proposed State Park Office Renovation and associated pavements at 834 Edgewater Drive, St. Marys, Ohio. The project site consisted of a single-story, slab-on-grade building with an entry canopy, U-shaped loop drive, and a small, 13-car parking lot along the inner edge of the loop drive. The Site Location Map is shown in Plate 1, attached to this report.

This study was conducted in accordance with Verdantas' Proposal No. 30131 dated October 22, 2024. Authorization to proceed was granted via a signed proposal dated January 7, 2025.

This report summarizes our understanding of the proposed construction, describes the investigative and testing procedures, presents the findings, discusses our evaluations and provides our design and construction recommendations for the proposed improvements.

The purpose of this investigation was to evaluate the subsurface conditions and laboratory data relative to the design and construction of foundations, floor slabs, and associated pavements for the referenced site. This investigation included six (6) test borings, field and laboratory soil testing, and a geotechnical engineering evaluation of the test results.

This report includes:

- A description of the existing subsurface soils, rock, and groundwater conditions encountered in the borings,
- Laboratory test results performed on selected soil samples,
- Design recommendations for building foundations, floor slab, and pavements,
- Preliminary recommendations concerning soil and rock, and groundwater-related construction procedures such as site preparation, earthwork, pavement subgrade preparation, and related field testing.

2. INVESTIGATIVE PROCEDURES

This subsurface investigation included six (6) test borings, designated as Borings B-1 through B-6, drilled by EnviroCore, Inc. on May 1, 2025. EnviroCore, Inc. was contracted by Verdantas for drilling services, and field operations were completed under the coordination of a Verdantas management team. The test borings were located in the field by the driller per the provided boring location plan using a handheld Global Positioning System (GPS) device and are presented on the boring location plan attached as Plate 2.0. GPRS LLC, a private utility locate service, was contracted to delineate subsurface utilities within a 10-foot radius of each borehole location prior to the commencement of drilling operations. Boring B-3 was moved 7 feet west of the originally staked location to avoid damage to the water line detected by GPRS equipment.

The test borings were performed in general accordance with geotechnical investigative procedures outlined in ASTM Standards D 1452 and D 5434. The test boreholes performed during this investigation were drilled with a track drilling rig utilizing 3¼-inch diameter hollow-stem augers. All boring surface elevations and termination depths are presented in **Error! Reference source not found.**.. Note that ground elevations were obtained using Google Earth and considered approximate.

Table 1: Borehole Surface and Termination Elevations			
Borehole No.	Surface Elevation (ft)	Termination Depth (ft)	Termination Elevation (ft)
B-1	878	20	858
B-2	880	20	860
B-3	879	20	859
B-4	879	20	859
B-5	880	10	870
B-6	877	10	867

During auger advancement, soil samples were collected at 2½-foot intervals to a depth of 10 feet and at 5-foot intervals thereafter. Split-spoon (SS) samples were obtained by the Standard Penetration Test (SPT) Method (ASTM D 1586), which consists of driving a 2-inch outside diameter split-barrel sampler into the soil with a 140-pound weight falling freely through a distance of 30 inches. The sampler was driven in three successive 6-inch increments with the number of blows per increment being recorded. The sum of the number of blows required to advance the sampler the second and third 6-inch increments is termed the Standard Penetration Resistance (N-value) and is presented on the Logs of Test Borings attached to this report. The soil samples were sealed in jars and transported to our laboratory for further classification and testing.

Soil conditions encountered in the test borings are presented in the Logs of Test Borings, along with information related to sample data, SPT results, water conditions observed in the borings, and laboratory test data. It should be noted that these logs have been prepared on the basis of laboratory classification and testing as well as field logs of the encountered soils.

All samples of the subsoils were visually or manually classified using the Unified Soil Classification System (ASTM D 2487 and D 2488) and were tested in our laboratory for moisture content (ASTM D 2216). Atterberg limits tests (ASTM D 4318) and particle size analyses (ASTM D 422) were performed on selected samples to determine soil classification and index properties. These test

results are presented on the Logs of Test Borings, Tabulation of Test Data sheets, and the Grain Size Distribution sheets attached to this report.

Experience indicates that the actual subsoil conditions at a site could vary from those generalized on the basis of test borings made at specific locations. Therefore, it is essential that a geotechnical engineer be retained to provide soil engineering services during the site preparation, excavation, and foundation phases of the proposed project. This is to observe compliance with the design concepts, specifications, and recommendations, and to allow design changes in the event subsurface conditions differ from those anticipated prior to the start of construction.

3. PROPOSED CONSTRUCTION

It is our understanding that the project consists of the construction of a new single-story, slab-on-grade building with an entry canopy on the building's east side. The structure is planned to be supported on a shallow foundation system. Structural loads were not available at the time of this report and are assumed to be light in magnitude. Maximum column and wall loads are assumed to be 100 kips and 3,000 pounds per lineal foot (plf), respectively.

Pavements are anticipated to consist of flexible asphalt pavements. Final site elevations are anticipated to approximate existing site elevations.

4. GENERAL SITE AND SUBSURFACE CONDITIONS

4.1 General Site Conditions

The project site is to the north of Grand Lake within Grand Lake St. Marys State Park, west of the City of St Marys in Auglaize County, Ohio. It is approximately 1.4 miles southwest of the intersection of SR 29 and US Route 33. According to a Google Earth image from October 29, 2024, the site features grass-covered ground with few mature trees in the southern portion and dense woodland in the northern portion between existing drives. The site is accessible via Edgewater Drive from the east.

During the subsurface exploration, a topsoil layer with a thickness of approximately 12 to 14 inches was encountered. Fine-grained native cohesive soil was observed throughout the investigated depths. No evidence of fill material or rock formations was detected.

4.2 Regional Geology

The subject site is located within the Central Lowland physiographic region of Ohio. This area is characterized by relatively flat terrain with gentle rolling hills and is part of the larger Till Plains section, which includes glacial deposits from the last Ice Age.

The region around St Marys, Ohio, is covered by Wisconsin-age glacial till. These deposits consist of clay, silt, sand, and gravel, which were left behind by retreating glaciers. The till is typically high in lime content and varies in thickness across the area.

The bedrock underlying this area is primarily composed of carbonate rocks, including limestone and dolomite. These rocks are part of the Lower Paleozoic era and provide a stable foundation for construction and other land-use activities. The bedrock in this area is between Elms. 720 to 740. Ground elevation is 789±, so the bedrock depth is between 139 to 159 feet below the grade.

Based on the available online ODNR database, no mining or probable karsts are indicated for the project site. The closest identified Suspect Carbonate Karst is located 2.5± miles southeast of the subject site. The closest identified mining area is about 4.7± miles northeast of the site.

4.3 General Subsurface Conditions

The results of our field and laboratory tests indicate that the subsoils primarily consist of medium stiff to very stiff cohesive soils. A soft zone of cohesive soil was found below the surfacing topsoil in Boring B-2 (SS-1), B-3 (SS-1), and B-4 (SS-1). In Boring B-3 (samples SS-2 and SS-3), very loose granular soils were identified, which is an exception to the typical findings. No apparent fill was detected. Additional descriptions of the soil stratigraphy encountered in the borings are detailed in the Logs of Test Borings attached to the report in Appendix A.

The subsurface soils encountered predominantly consist of medium stiff to very stiff native cohesive soils with a random soft zone, such as one in Boring B-2. Based on visual-manual identification and lab testing, these soils are classified as lean clays (CL), sandy silty clays (CL-ML), and sandy silts (ML), Silty Clays (CL-ML), and Sandy lean Clays (CL). These soils were encountered underlying the surface material in all borings. Various amounts of sand and gravel were detected in all borings, and traces of organics were noted in samples close to the surface. Standard Penetration Test (SPT) N-values were on the order of 4 to 29 blows per foot (bpf). Unconfined compressive strength (UCS) ranged from 4,000 to 9,000 pounds per square foot (psf). Moisture contents ranged from 13 to 22 percent.

Liquid limits of 36, 41, and 35 percent and plasticity indices of 17, 21, and 17 percent were determined from samples collected in borings B-2 (SS-1), B-5 (SS-1), and B-6 (SS-2), respectively. These values, along with gradation results, are indicative of Lean clay (CL), in accordance with USCS designations and A-6b, and A-7-6 in accordance with the AASHTO ODOT soil Classification System.

Granular native soil consisted of very loose Clayey sand (SC) and Silty Sand (SM) were encountered in Boring B-3 (SS-2 and SS-3). Standard Penetration Test (SPT) N-values were on the order of 3 to 4 blows per foot (bpf). Moisture contents ranged from 21 to 22 percent.

Bedrock was not encountered in the borings.

4.4 Groundwater Conditions

No groundwater was encountered during the drilling operations in any of the borings. Upon completion, all boreholes were observed to be dry except for the boring B-3, where the groundwater level was six feet below the grade. The boreholes collapsed at depths ranging from 7 to 16 feet below grade. It is important to note that each borehole was drilled and backfilled on the same day, without the installation of instrumentation to monitor long-term groundwater levels.

It should be noted that groundwater elevations can fluctuate with seasonal and climatic influences. In particular, “perched” water may be encountered in the crushed stone or existing fill materials that are underlain by the relatively impermeable native cohesive soils, within cobble zones, as well as at the soil/bedrock interface. Therefore, the groundwater conditions may vary at different times of the year from those encountered during this investigation.

Based on the soil characteristics and groundwater conditions encountered in the borings, it is our opinion that the static, long-term groundwater table is about 6 feet below grade. However, groundwater can be encountered elsewhere when it seeps from the surface through the fill and gets trapped on the top of the less permeable cohesive soils, at the delineation border between upper granular soils underlined with cohesive soils, or when it’s trapped at the top of solid bedrock which is the case throughout the borings drilled for this project based on the field recorded water depths during the drilling process. Generally, groundwater elevations can fluctuate with seasonal and climatic influences. In particular, “perched” groundwater may be encountered within the pavement base materials, fill materials, and in the granular soils (liquefier) sandwiched in between less permeable cohesive strata. Therefore, the groundwater conditions may vary at different times of the year from those encountered during this exploration.

5. DESIGN RECOMMENDATIONS

The following conclusions and recommendations are based on the data obtained during the field investigation and our understanding of the proposed construction. If the project information or location as outlined is incorrect or should change significantly, a review of these recommendations should be made by VDT. Additionally, these recommendations are subject to additional geotechnical exploration and analysis for final design and are also contingent on satisfactory completion of the recommended site and subgrade preparation and fill placement operations described in Section 6.0, "Construction Recommendations."

5.1 Building Foundations

5.1.1 Shallow Building Foundations

Final site grades are anticipated to approximate existing site grades. Foundations are anticipated to bear approximately 3.5 feet below finished exterior grades (minimum required depth below finished exterior grades for protection from frost penetration). Interior foundations in heated structures may bear slightly shallower than this depth.

Based on the results of the field and laboratory testing conducted for the borings associated with this investigation, the soils encountered at the anticipated foundation bearing depth primarily consist of medium stiff to very stiff native cohesive soil. These medium stiff and very stiff layers are generally considered suitable materials for supporting the proposed structure, provided that any softer or weaker layers are replaced with suitable foundation material.

However, Boring B-3 revealed a section of very loose granular soil at the foundation bearing elevation. These soils are not considered suitable for foundation support and will required in place densification using a hoe pac or similar equipment or removed and replaced. It should be noted that perched water was encountered in this material that may make in place densification difficult and undercutting and replacement may be required. In this case consideration should be given to utilizing an open graded stone similar to ASTM No. 57 as backfill for the undercut due to the potential for groundwater.

If unsuitable foundation soils or soils that cannot be suitably modified in place are encountered, over-excavation should extend through these materials to suitable bearing soils. The base of the over-excavation should be widened one foot for every foot of depth and centered along the footing. The over-excavated areas should be backfilled with dense-graded aggregate, placed in controlled lifts, and compacted to not less than 100 percent of the maximum dry density as determined by ASTM D 698 (Standard Proctor) or open graded stone discussed above. Alternatively, the over-excavated areas could be backfilled with lean concrete having a minimum compressive strength of 1,500 pounds per square inch (psi) or other flowable controlled-density fill having a minimum compressive strength of 300 psi. If foundations will be placed at the base of the over-excavation or the lean concrete fill option will be utilized, widening the footing over-excavation will not be required. If the controlled-density fill option is utilized, the footing over-excavation shall be widened as discussed above.

Following the satisfactory completion of the site preparation and footing excavation inspections outlined in this report, the proposed structure(s) may be supported on conventional shallow foundation systems designed utilizing a net allowable bearing pressure of 2,000 pounds per square foot (psf). In using a net allowable soil pressure, the weight of the footings, backfill over the footings, or floor slabs need not be included in the structural loads for dimensioning footings.

The bearing materials should be field verified as native cohesive soils with a minimum unconfined compressive strength of 2,000 psf for cohesive soils or granular soils suitably modified in place to meet the compaction/bearing criteria presented below, or properly placed and compacted new engineered fill.

Suitable compaction/bearing of granular foundation soils can be verified as:

- Exhibiting a compacted (in-situ) dry density of at least 100 percent of the maximum dry density determined by Standard Proctor (ASTM D 698) laboratory compaction,
- A dynamic cone penetrometer (DCP) reading of at least 8 blows per increment (average over three increments), or
- Other methods to demonstrate an equivalent SPT N-value of 10 bpf or greater.

We strongly recommend that the bearing surface at the bottom of all footing excavations be inspected during construction by a VDT geotechnical engineer or qualified representative. Inspection should be performed to verify that the exposed soil conditions at the bearing elevations are consistent with the subsurface conditions encountered in the test borings and are suitable for foundation bearing. Additionally, the presence of our engineer will help facilitate the timely remediation of unsuitable soil conditions. If the results of hand penetrometer or other strength tests indicate the exposed soil conditions are not suitable for the design bearing pressure, it may be necessary to increase the footing size to accommodate the lower bearing strengths or to over-excavate and backfill with engineered fill or flowable fill.

All exterior footings and footings in unheated areas should be constructed at a minimum frost penetration depth of 3.5 feet below finished exterior grades. Interior foundations may bear shallower on suitable bearing soils as described above, or on properly placed and compacted new engineered fill. Wall (strip) footings should be at least 18 inches wide, and column footings should be at least 30 inches wide, regardless of size based on design loads and the allowable bearing pressure.

Utilizing the above allowable bearing pressures and proper foundation inspection techniques, the total settlement may be on the order of 1 inch, and differential settlement should not exceed $\frac{3}{4}$ of the total settlement..

5.2 Subgrades

5.2.1 Existing Subgrade

The subgrades that would result upon the satisfactory completion of the site preparation as described in Section 6.0 of this report are considered marginally suitable for support of the proposed floor slab and the pavement. Based on field and laboratory data developed during this investigation, the subgrade soils consist of cohesive soils. Laboratory analyses for Borings B-2 (SS-1), B-5 (SS-1), and B-6 (SS-2) as well as visual descriptions of the upper profile, indicate that the cohesive subgrade soils may be generally classified as A-6b, and A-7-6 in accordance with the Ohio Department of Transportation (ODOT) system of soil classification. These cohesive soils are considered fair to poor as subgrade materials because they have relatively low permeabilities and a high percentage of silt and clay particles, which makes them susceptible to moisture, frost penetration, and frost heave.

At the time of this investigation, moisture contents in the upper 5 feet of the cohesive subgrade soils ranged from approximately 15 to 24 percent for the A-6b and A-7-6 soils. These moisture contents are estimated to be near to significantly above optimum moisture content for these soils

as per Ohio Geotechnical Design Manual Section 600. Remedial action should be anticipated to be required to adjust the moisture contents of the existing materials and achieve proper compaction of the subgrade.

5.2.2 Modified Subgrade

Although not anticipated to be prevalent, if soils are dry of optimum, water should be uniformly mixed into the subgrade. More likely to be encountered at this site are soils that are wet of the optimum. Where soils wet of optimum are encountered, lowering the moisture content by scarification and aeration (discing and exposure to sun and wind) may be required. Very moist to wet soils will “pump” under the operation of heavy equipment, resulting in deep rutting and perhaps rendering the operation of grading and paving equipment difficult or impossible.

If the schedule does not allow for scarification and aeration of the soils, other methods of subgrade modification may be required in areas of high moisture content. Modification may be achieved by undercutting and replacement with granular subbase (possibly in combination with a geotextile separation layer or geogrid reinforcement), mixing stone into the subgrade, or treating the subgrade with lime or cement. The method of subgrade modification should be determined at the time of construction (See Section 6, “Construction Recommendations - Site and Subgrade Preparation”).

5.3 Floor Slabs

It is recommended that all floor slabs be “floating,” that is, fully ground supported and not structurally connected to walls or foundations. This is to reduce the possibility of cracking and displacement of the floor slabs because of differential movements between the slab and the foundation. Such movements could be detrimental to slabs that are rigidly connected to the foundations. There may be certain areas where it will be difficult or impractical to make the slab floating. In such areas, it may be necessary to increase the slab thickness and reinforcement to prevent the foundation from cracking the slab and settling independently.

For properly prepared floor slab subgrade soils, a modulus of subgrade reaction (k) on the order of 135 pounds per cubic inch (pci) should be utilized for design. It is recommended that the floor slab be supported on a minimum 6-inch layer of relatively clean granular material such as sand and gravel or crushed stone. This is to help distribute concentrated loads and provide more uniform subgrade support beneath the slab.

5.4 Groundwater Control and Drainage

The groundwater conditions encountered in the borings during this investigation are summarized in Section 4.4. Based on the soil characteristics and groundwater conditions encountered during this investigation, it is our opinion that the “normal” groundwater table can generally be expected at depths on the order of 6 feet or greater below existing grade.

It is our experience that adequate control of groundwater seepage or surface water run-off into excavations which do not extend more than a few feet below the ambient groundwater table should be achievable by minor dewatering systems, such as pumping from prepared sumps.

If excessive seepage is experienced during construction, VDT should be notified to evaluate other dewatering methods as per site conditions.

5.5 Flexible (Asphalt) Pavement

Based on the results of the laboratory testing, as well as visual classification of the recovered samples, we recommend a CBR value of 5 percent. This CBR value is based on subgrade compacted to at least 100 percent of the maximum dry density as determined by ASTM D 698 (Standard Proctor) or verified as stable through proof rolling.

It should be noted that we are not privy to the design traffic loads or intended design life, and the subgrade support recommendations indicated herein should be reviewed by the site engineer in conjunction with the design traffic criteria to determine the required pavement sections. In any case, we recommend that the light-duty pavement cross-section consist of at least 3 inches of asphalt underlain by 6 inches of aggregate base for even the lightest-duty pavements based on our experience regarding environmental exposure and reasonable serviceability. For the same reason, we recommend that the heavy-duty pavement cross-section consist of at least 4 inches of asphalt underlain by 8 inches of aggregate base.

All paving operations should conform to Ohio Department of Transportation (ODOT) specifications. The pavement and subgrade preparation procedures outlined in this report should result in a reasonably workable and satisfactory pavement. It should be recognized, however, that all flexible pavements need repairs or overlays from time to time as a result of progressive yielding under repeated traffic loads for a prolonged period of time, as well as exposure to weather conditions.

5.6 Rigid (Concrete) Pavement

For properly prepared subgrade soils, a modulus of subgrade reaction (k) of 135 pounds per cubic inch (pci) may be used for rigid pavement design. A concrete pavement section is recommended in the loading-unloading areas, areas of repetitive turning, site exit and entrance aprons, and trash enclosure areas (including where the truck rests while servicing the container). This section should consist of a minimum of 6 inches of reinforced, air-entrained concrete with a minimum compressive strength of 3,500 pounds per square inch (psi) underlain by a minimum of 6 inches of a dense-graded aggregate base such as ODOT Item 304. The pavement section should be supported on subgrade compacted to at least 100 percent of the maximum dry density as determined by ASTM D 698 (Standard Proctor) or verified as stable through proof rolling. All paving operations should conform to the State of Ohio Department of Transportation (ODOT) specifications.

5.7 Seismic Considerations

We have reviewed seismic design parameters in accordance with ASCE 7-10, with respect to the conditions encountered in the borings.

Laboratory test results identified cohesionless soil layers ($PI < 20$) for which the Nch or N-method is considered. The average SPT N_{60} -value (Nch) was calculated to be on the order of 17 bpf with an average hand penetrometer value of 3,600 psf, and an average undrained shear strength (s_u) of 3,250 psf. It should be noted that the boring termination depth is 20 feet, which is less than the ASCE 7-10 recommended depth of 100 feet to determine the seismic class. Based on the average SPT- N_{60} and hand penetrometer values for the upper 20 feet, the soil class is identified as Class D – Stiff Soil.

5.8 Excavations and Slopes

The sides of temporary excavations for building foundations, utility installations, and other construction should be adequately sloped to provide stable sides and safe working conditions. Otherwise, the excavation must be properly braced against lateral movements. In any case, applicable Occupational Safety and Health Administration (OSHA) safety standards must be followed.

Based on the test borings, it is likely that excavations will encounter a range of soil conditions that include the following OSHA designations:

- Type A soils (cohesive soils with unconfined compressive strengths of 3,000 pounds per square foot (psf) or greater),
- Type B soils (cohesive soils with unconfined compressive strengths greater than 1,000 psf but less than 3,000 psf, weathered rock), and
- Type C soils (granular soils).

For temporary excavations in Type A, B, and C soils, side slopes must be no steeper than $\frac{3}{4}$ horizontal to 1 vertical ($\frac{3}{4}$ H:1V), 1H:1V, and 1½H:1V, respectively. For situations where a higher strength soil is underlain by a lower strength soil and the excavation extends into the lower strength soil, the slope of the entire excavation is governed by that required for the lower strength soil. In all cases, flatter slopes may be required if lower strength soils or adverse seepage conditions are encountered during construction.

For permanent excavations and slopes, we recommend that grades be no steeper than 3H:1V without a more extensive geotechnical evaluation of the proposed construction plans and site conditions.

6. CONSTRUCTION RECOMMENDATIONS

6.1 Site and Subgrade Preparation

Prior to proceeding with construction operations, all structures, pavements, vegetation, topsoil, root mat, and other deleterious non-soil materials should be removed from the proposed construction areas. Suitable topsoil may be stockpiled for later use in landscaped areas. Topsoil surface material thicknesses are discussed in Section 4.1 and may vary across the site, and from the thickness indicated at the boring locations.

Dark soils having the appearance of topsoil but exhibiting only root “hairs” or trace organics less than approximately five percent, may not require stripping for the full depth of the darkly colored zone, provided the subgrade can be satisfactorily proof rolled as described below. The actual amount of required stripping should be determined in the field by a geotechnical engineer or qualified representative.

Upon completion of stripping and clearing, the areas intended to support floor slabs, pavements, and new fill should be carefully inspected by a geotechnical engineer. At that time, the engineer should observe proof rolling of the cohesive subgrade soils using a minimum 20-ton loaded truck or other pneumatic-tired vehicle of similar size and weight. The truck should make a minimum of two passes covering the proposed development area, with additional passes as necessary to achieve required compaction and/or subgrade stabilization.

The purpose of the proof-rolling operations for the cohesive soils is to locate any soft or excessively wet soils present at the time of construction. Any unsuitable materials observed during the inspection and proof-rolling operations should be undercut and replaced with compacted engineered fill or stabilized in place utilizing conventional remedial measures such as discing, aeration, and recompaction. Once the site has been proof rolled, inspected, and stabilized, the proof-rolled or inspected subgrades should not be exposed to wet conditions. It should be recognized that during periods of wet weather, the silty and clayey soils that will be exposed at design subgrades will tend to pond water for short periods of time, with the potential to deteriorate the prepared subgrade.

Any unsuitable materials observed during the inspection and proof-rolling operations should be undercut and replaced with compacted fill or stabilized in place utilizing conventional remedial measures such as discing, aeration, and recompaction. Based on the variable fill materials present at the site, some remedial action should be anticipated. Once the site has been proof rolled, inspected, and stabilized, the proof rolled or inspected subgrades should not be exposed to wet conditions. It should be recognized that during periods of wet weather, the clayey soils that will be exposed at design subgrades will tend to pond water for short periods of time, with the potential to deteriorate the prepared subgrade.

The results of the inspection and proof-rolling operations will be partially dependent on construction operations, the moisture content of the soil, and the weather conditions prevalent at the time. If pumping or rutting is encountered and difficulty is experienced in the operation of construction equipment, VDT should be notified in order to determine which method of subgrade modification may be best suited for the conditions encountered. Should such conditions be experienced, we may recommend that a small test area be used to determine the necessary depth of undercutting and stone replacement or other remedial action necessary to achieve a stable subgrade condition.

6.2 Fill

Material for engineered fill or backfill required to achieve design grades may consist of any non-organic soils having a maximum dry density as determined by the Standard Proctor (ASTM D 698) greater than 90 pounds per cubic foot (pcf) and exhibiting a liquid limit of less than 50 percent. On-site soils may be used as engineered fill materials provided that they are free of organic matter, debris, excessive moisture, and rock or stone fragments larger than 3 inches in diameter. Depending on seasonal conditions, the on-site soils may be wet of optimum and could require scarification and aeration to achieve satisfactory compaction. Additionally, the moisture content of some on-site soils may need to be increased with uniformly mixing of water into the materials. If the construction schedule does not allow for scarification and aeration activities or moisture addition activities, it may be more practical or economical to utilize imported granular fill.

Fill should be placed in uniform layers no more than 8 inches thick (loose measure) and adequately keyed into stripped and scarified soils. All fill within the building areas and pavement subgrades should be compacted to not less than 100 percent of the maximum dry density as determined by ASTM D 698 (Standard Proctor).

Based on the preliminary investigation borings, the upper soil profile at the site consists of native cohesive soils. The contractor should be prepared to use a sheepsfoot roller to provide effective compaction of the cohesive soils. If granular soils are utilized as engineered fill, a vibratory smooth-drum roller should be utilized for compaction. Vibratory compaction should not be utilized within 10 feet of existing structures. Rather, additional passes using the static mode should be used in these areas. In narrow utility or footing excavations, the on-site cohesive soils may be difficult to compact; therefore, a clean granular material may be required in these areas.

Scarified subgrade soils and all fill material should be within 3 percent of the optimum moisture content to facilitate compaction. Furthermore, fill material should not be frozen or placed on a frozen base. It is recommended that all earthwork and site preparation activities be conducted under adequate specifications and properly monitored in the field by a qualified geotechnical testing firm.

6.3 Foundation Excavations

As mentioned in Section 5.1, foundation excavations should have a detailed inspection performed for each foundation. A geotechnical engineer or qualified representative should perform these inspections to verify that the exposed materials are similar to those encountered in the borings and are suitable for foundation bearing and/or new engineered fill has been properly placed and compacted.

We recommend that the foundation excavations be concreted as soon as practical after they are excavated, and that water not be allowed to pond in any excavation. If it is necessary to leave the bearing surface open for any extended period of time, we recommend that a thin mat of lean concrete be placed over the bottom of the excavation to reduce damage to the surface from weather or construction. Foundation concrete should not be placed on frozen or saturated subgrade.

7. QUALIFICATION OF PRELIMINARY RECOMMENDATIONS

Our evaluation of foundation, floor slab, and pavement geotechnical design and construction conditions has been based on our understanding of the site and project information and the data obtained during our field investigation. The general subsurface conditions were based on interpretation of the subsurface data at specific boring locations. When the final structure locations become available, additional geotechnical exploration in the area of the proposed development should be performed. The findings of such an investigation will be presented in a supplemental report. Based on the results of the final design investigation, the recommendations of this report will be reviewed and modified, as necessary.

Regardless of the thoroughness of a subsurface investigation, there is the possibility that conditions between borings will differ from those at the boring locations, that conditions are not as anticipated by the designers, or that the construction process has altered the soil conditions. Therefore, experienced geotechnical engineers should observe earthwork and foundation construction to confirm that the conditions anticipated in design are noted. Otherwise, VDT assumes no responsibility for construction compliance with the design concepts, specifications, or recommendations.

The nature and extent of variations between the borings may not become evident until the course of construction. If such variations are encountered, it will be necessary to reevaluate the recommendations of this report and the final geotechnical subsurface investigation report after on-site observations of the conditions.

Our professional services have been performed, our findings derived, and our preliminary recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties, either expressed or implied. VDT is not responsible for the conclusions, opinions, or recommendations of others based on this data.

Plates

Plate 1.0	Site Location Map
Plate 2.0	Test Boring Location Plan



**Approximate
Site Location**

SITE LOCATION MAP
GLSM STATE PARK OFFICE RENOVATION
834 EDGEWATER DRIVE,
ST.MARYS OHIO

PREPARED FOR
BUEHRER GROUP
314 CONANT STREET
MAUMEE, OHIO

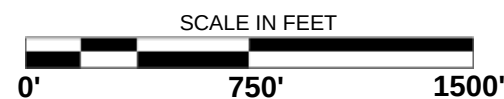
DRAWN: NT/ 05/05/2025

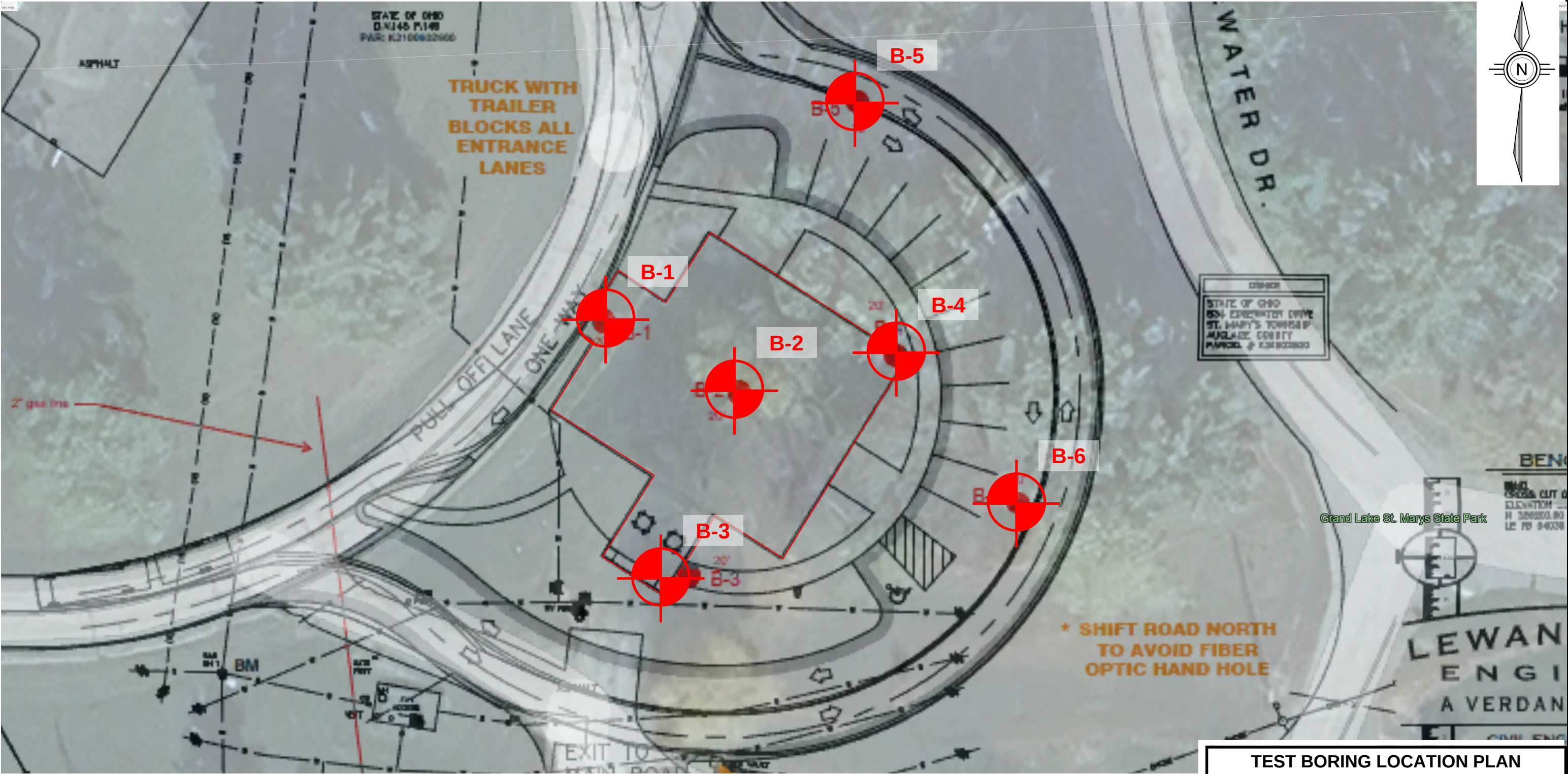
REVISED: ---

PROJECT No: 30131

verdantas

PLATE 1.0





LEGEND:



BASE PLAN "GRAND LAKE ST. MARYS STATE PARK-LAYOUT1" PROVIDED BY LEWANDOWSKI ENGINEERS A VERDANTAS COMPANY.

TEST BORING LOCATION PLAN
GLSM STATE PARK OFFICE RENOVATION
834 EDGEWATER DRIVE,
ST.MARYS OHIO

PREPARED FOR
BUEHRER GROUP
314 CONANT STREET
MAUMEE, OHI

DRAWN: NT/05/05/2025

REVISED: ---

PROJECT No: 30131

PLATE 2.0



Figures

Logs of Test Borings

Legend Key

Tabulation of Test Data

Laboratory Test Results



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BORING NUMBER B-1

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	None
		AT END OF DRILLING	None
		0hrs AFTER DRILLING	Backfilled w/Cuttings and Bentonite Chips

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 12 Inches						
			1.0'						
			Moist Medium Stiff Brown LEAN CLAY w/Sand and Trace Organics (CL)	SS 1	67	2-3-4 (7)	4.25		24
			3.5'						
			Moist Very Stiff Brown SANDY SILTY CLAY w/Trace Gravel (CL-ML)	SS 2	78	6-9-11 (20)	4.50		15
5									
				SS 3	100	7-9-14 (23)	4.53	111	15
			8.5'						
			Moist Very Stiff Brown SANDY SILT w/Trace Gravel (ML)	SS 4	100	6-10-13 (23)	4.50		13
10									
			13.5'						
			Moist Stiff Gray LEAN CLAY w/Sand and Trace Gravel (CL)	SS 5	100	4-6-8 (14)	4.00		25
15									
			20.0'	SS 6	89	3-4-5 (9)	2.75		20
20			Bottom of hole at 20.0 feet.						



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BORING NUMBER B-2

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	
		None	
		AT END OF DRILLING	
		None	
		0hrs AFTER DRILLING	
		Backfilled w/Cuttings and Bentonite Chips	

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 12 Inches						
			1.2'	SS 1	67	1-2-2 (4)	2.25		22
			3.5'	SS 2	78	1-3-6 (9)	3.46	112	15
5			@6': Very Stiff, Gray/Brown	SS 3	89	5-8-10 (18)	4.50		16
			8.5'	SS 4	100	6-10-11 (21)	4.50		17
10			13.5'	SS 5	94	5-7-7 (14)	4.50		20
15			18.5'	SS 6	100	3-3-5 (8)	2.50		22
20			20.0'						
			Bottom of hole at 20.0 feet.						



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BORING NUMBER B-3

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	
		None	
		▼ AT END OF DRILLING	
		6.0 ft	
		0hrs AFTER DRILLING	
		Backfilled w/Cuttings and Bentonite Chips	

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 12 Inches						
			1.0'						
			Moist Soft Brown SANDY LEAN CLAY w/Trace Gravel and Organics (CL)	SS 1	72	1-2-2 (4)	2.25		20
			3.5'						
			Moist Very Loose Brown CLAYEY SAND w/Trace Gravel and Organics (SC)	SS 2	78	2-2-2 (4)	NP		21
5			6.0'						
			Moist Very Loose Brown SILTY SAND w/Trace Gravel (SM)	SS 3	89	1-1-2 (3)	NP		22
			8.5'						
			Moist Stiff Brown LEAN CLAY w/Sand and Trace Gravel (CL)	SS 4	72	2-4-6 (10)	1.66	99	23
10									
			13.5'						
			Moist Very Stiff Gray SANDY SILTY CLAY w/Trace Gravel (CL-ML)	SS 5	100	4-7-9 (16)	4.00		16
15									
			18.5'						
			Moist Stiff Gray LEAN CLAY w/Sand and Trace Gravel (CL)	SS 6	100	3-4-5 (9)	2.00		18
20			20.0'						
			Bottom of hole at 20.0 feet.						



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BORING NUMBER B-4

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	
		None	
		AT END OF DRILLING	
		None	
		0hrs AFTER DRILLING	
		Backfilled w/Cuttings and Bentonite Chips	

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 12 Inches						
			1.0'						
			Moist Soft Brown LEAN CLAY w/Sand (CL)	SS 1	78	0-2-2 (4)	2.25		24
			3.5'						
			Moist Very Stiff Brown/Gray SANDY SILTY CLAY w/Trace Gravel and Iron Oxide Stain Seam (CL-ML)	SS 2	78	0-5-8 (13)	4.46	111	15
5			@6': Gray/Brown	SS 3	89	8-12-13 (25)	4.50		16
			@8.5': w/Trace Organics	SS 4	100	7-12-17 (29)	4.50		18
10									
			13.5'						
			Moist Stiff Gray LEAN CLAY w/Sand and Trace Gravel (CL)	SS 5	100	5-5-7 (12)	2.17	118	16
15									
			18.5'						
			Moist Medium Stiff Gray LEAN CLAY w/Sand and Trace Gravel (CL)	SS 6	89	3-4-3 (7)	2.75		21
20			20.0'						
			Bottom of hole at 20.0 feet.						



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BORING NUMBER B-5

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	None
		AT END OF DRILLING	None
		0hrs AFTER DRILLING	Backfilled w/Cuttings and Bentonite Chips

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 14 Inches						
			1.2'	SS 1	78	2-3-6 (9)	2.75		20
			@3.5': Very Stiff, w/Sand	SS 2	89	3-7-10 (17)	2.75		21
	5		6.0'	SS 3	89	5-9-12 (21)	4.50		15
			@8.5': Gray/Brown, w/Dolomite Fragments	SS 4	100	7-10-12 (22)	4.50		15
	10		10.0'						
			Bottom of hole at 10.0 feet.						



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BORING NUMBER B-6

PAGE 1 OF 1

CLIENT	Bueher Group	PROJECT NAME	GLSM Park Office Renovation
PROJECT NUMBER	30131	PROJECT LOCATION	Saint Marys, OH
DRILLING CONTRACTOR	Enviro Core Scott HH	RIG NO.	B57
DRILLING METHOD	3-1/4 in. HSA	GROUND ELEVATION	
DATE STARTED	5/1/25	COMPLETED	5/1/25
LOGGED BY	SB	CHECKED BY	
NOTES			
GROUND WATER LEVELS:		AT TIME OF DRILLING	None
		AT END OF DRILLING	None
		0hrs AFTER DRILLING	Backfilled w/Cuttings and Bentonite Chips

ELEVATION (ft)	DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER	RECOVERY % (RQD)	BLOW COUNTS (N VALUE)	UNCONF. COMP. STR. (tsf)	DRY UNIT WT. (pcf)	PL 20 MC 40 LL 80 ▲ SPT N VALUE ▲
	0		TOPSOIL - 12 Inches						
			1.0'						
			Moist Medium Stiff Brown SANDY SILTY CLAY w/Trace Gravel (CL-ML)	SS 1	78	2-2-3 (5)	4.50		18
			3.5'						
			Moist Very Stiff Gray/Brown LEAN CLAY w/Sand and Trace Gravel (CL)	SS 2	67	3-7-12 (19)	4.50		16
5			@6': Brown	SS 3	100	5-8-10 (18)	4.50		16
				SS 4	100	6-10-10 (20)	4.50		16
10			10.0'						
			Bottom of hole at 10.0 feet.						

LEGEND KEY

Unified Soil Classification System Soil Symbols

	GW - WELL GRADED GRAVEL Includes Gravel-Sand mixtures, little or no fines.		GP - POORLY GRADED GRAVEL Includes Gravel-Sand mixtures, little or no fines.		GM - SILTY GRAVEL Includes Gravel-Sand-Silt mixtures.		GC - CLAYEY GRAVEL Includes Gravel-Sand-Clay mixtures.
	SW - WELL GRADED SAND Includes Gravelly Sands, little or no fines.		SP - POORLY GRADED SAND Includes Gravelly Sands, little or no fines.		SM - SILTY SAND Includes Sand-Silt mixtures.		SC - CLAYEY SAND Includes Sand-Clay mixtures.
	ML - SILT Includes Silt with Sand and Sandy Silt.		CL - LEAN CLAY Includes Sandy Lean Clay and Lean Clay with Sand and Gravel.		MH - ELASTIC SILT Includes Sandy Elastic Silt and Elastic Silt with Sand.		CH - FAT CLAY Includes Sandy Fat Clay and Fat Clay with Sand.
	CL-ML - SILTY CLAY Includes Clayey Silt of low plasticity.		OL - ORGANIC SILT and ORGANIC CLAY of low plasticity.		OH - ORGANIC SILT and ORGANIC CLAY of medium to high plasticity.		Pt - PEAT Includes humus, swamp and other soils with high organic content.
	FILL MATERIAL - Includes controlled and non-controlled soil and non-soil materials.		TOPSOIL		ASPHALT - Bituminous Asphalt		CONCRETE - Includes broken concrete rubble.
	Shale		Weathered Shale		Sandstone		Weathered Sandstone
	Coal						

Sample Symbols

	SS - Split Spoon		ST - Shelby Tube		RC - Rock Core		GS - Geoprobe Sleeve
			AU - Auger Cuttings		GB - Grab		

Notes:

- Exploratory borings were drilled on May 1, 2025, using 3¼- inch inside diameter hollow-stem augers.
- These logs are subject to the limitations, conclusions, and recommendations in the report and should not be interpreted separate from the report.
- The borings were located in the field by Enviro Core in accordance with boring location plans provided by Verdantas' design team.
- Unconfined Compressive Strength (tsf):
NP = Non-Plastic
NI = Not Intact



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SUMMARY OF LABORATORY RESULTS

PAGE 1 OF 1

CLIENT Bueher Group

PROJECT NAME GLSM Park Office Renovation

PROJECT NUMBER 30131

PROJECT LOCATION Saint Marys, OH

Borehole	Depth	Liquid Limit	Plastic Limit	Plasticity Index	Maximum Size (mm)	%<#200 Sieve	Class-ification	Water Content (%)	Dry Density (pcf)	Satur-ation (%)	Void Ratio
B-1	1.0							23.6			
B-1	3.5							15.2			
B-1	6.0							15.2	111.1		
B-1	8.5							12.6			
B-1	13.5							24.5			
B-1	18.5							19.6			
B-2	1.0	36	19	17	9.5	83	CL	22.4			
B-2	3.5							15.5	111.9		
B-2	6.0							15.8			
B-2	8.5							17.0			
B-2	13.5							20.3			
B-2	18.5							21.5			
B-3	1.0							19.7			
B-3	3.5							21.3			
B-3	6.0							22.0			
B-3	8.5							23.4	99.3		
B-3	13.5							16.4			
B-3	18.5							18.4			
B-4	1.0							23.9			
B-4	3.5							14.8	110.7		
B-4	6.0							16.3			
B-4	8.5							17.8			
B-4	13.5							15.7	117.9		
B-4	18.5							21.1			
B-5	1.0	41	20	21	9.5	85	CL	20.4			
B-5	3.5							20.8			
B-5	6.0							15.4			
B-5	8.5							15.4			
B-6	1.0							18.2			
B-6	3.5	35	18	17	12.5	83	CL	15.9			
B-6	6.0							15.7			
B-6	8.5							15.6			



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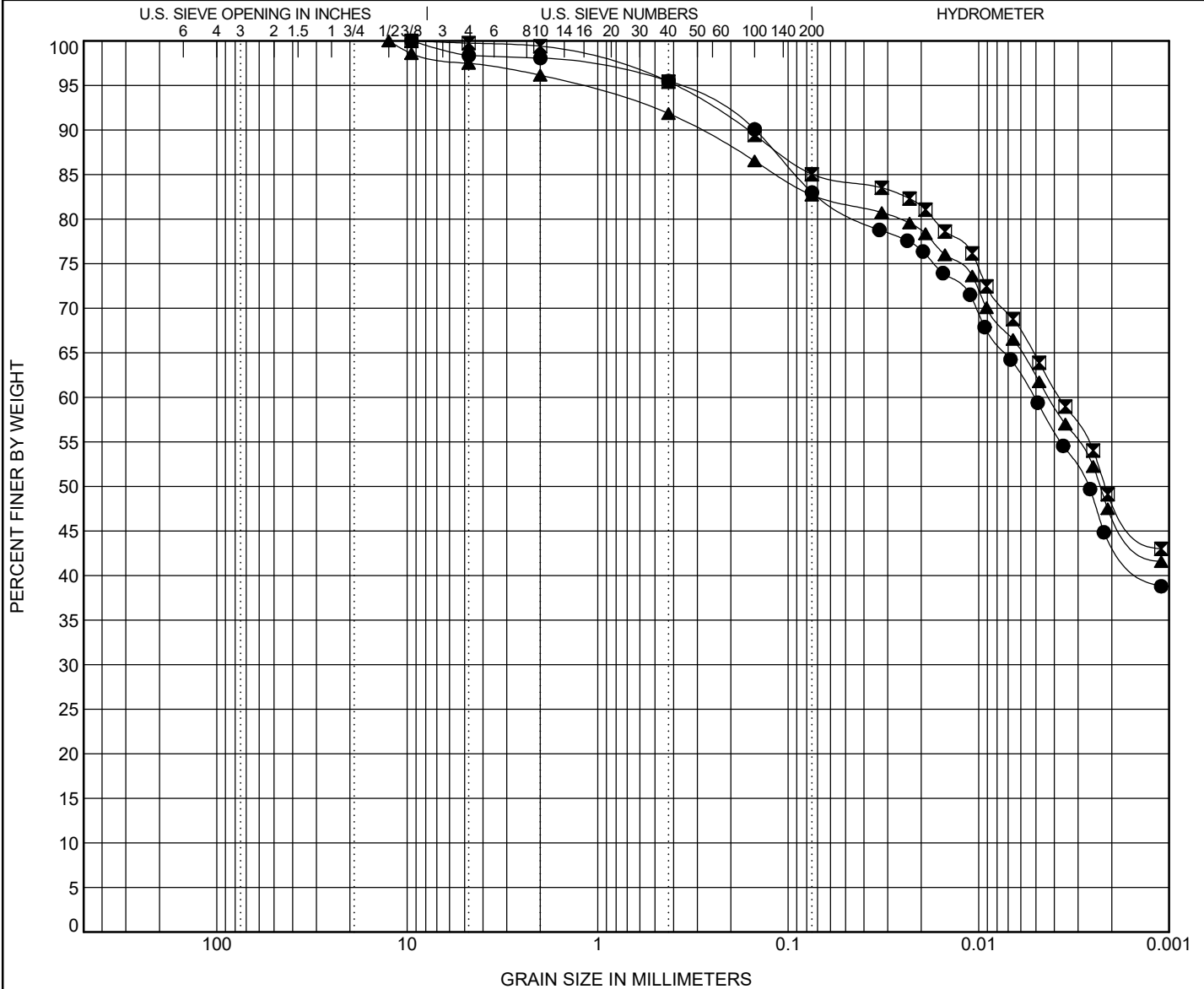
GRAIN SIZE DISTRIBUTION

CLIENT Bueher Group

PROJECT NAME GLSM Park Office Renovation

PROJECT NUMBER 30131

PROJECT LOCATION Saint Marys, OH



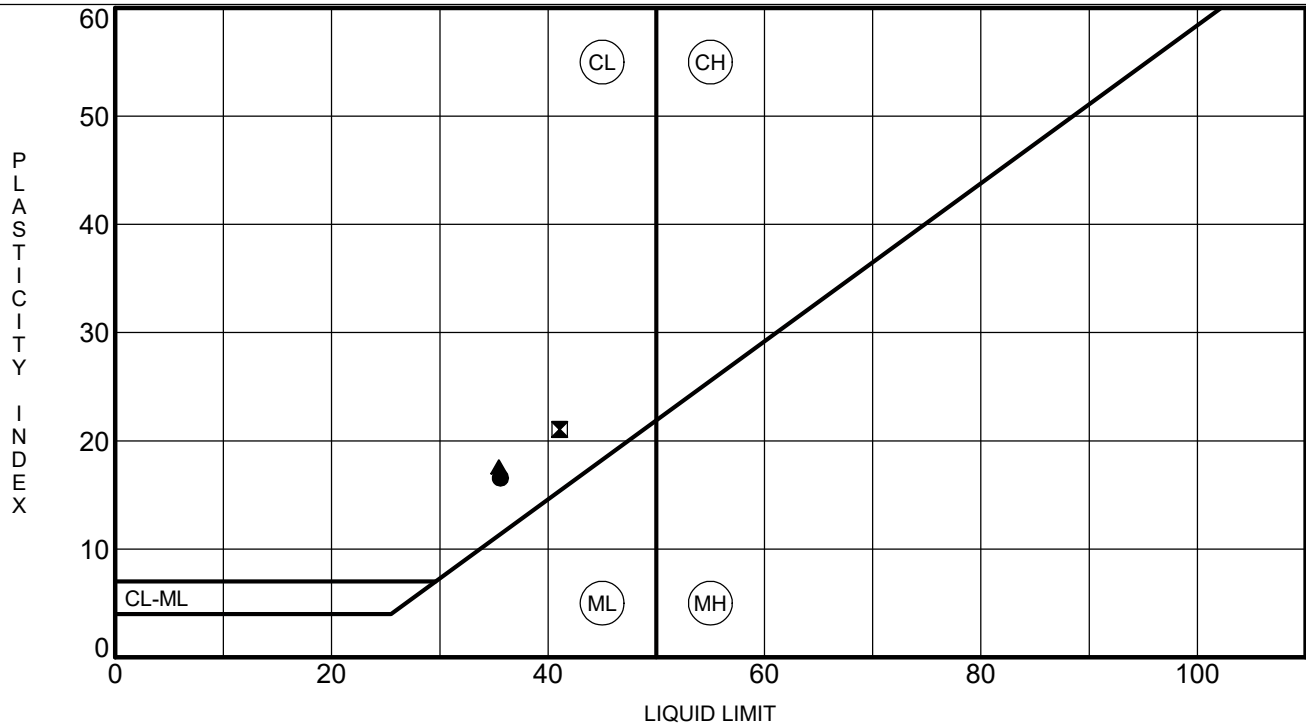
ATTERBERG LIMITS' RESULTS

CLIENT Bueher Group

PROJECT NAME GLSM Park Office Renovation

PROJECT NUMBER 30131

PROJECT LOCATION Saint Marys, OH

[illegible]



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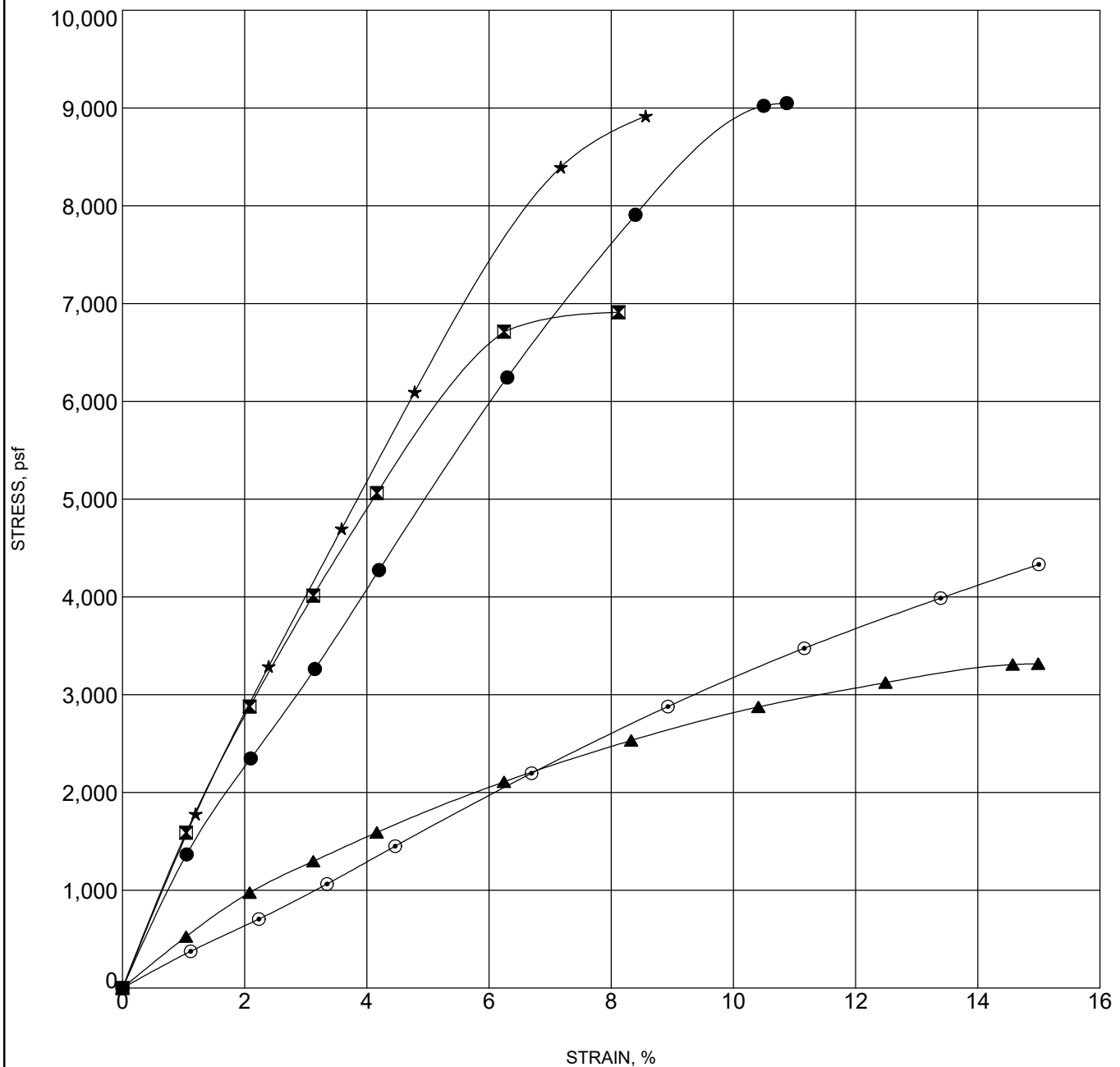
UNCONFINED COMPRESSION TEST

CLIENT Bueher Group

PROJECT NAME GLSM Park Office Renovation

PROJECT NUMBER 30131

PROJECT LOCATION Saint Marys, OH



Specimen Identification			Classification	γ_d	MC%
●	B-1	6.0		111	15
⊠	B-2	3.5		112	16
▲	B-3	8.5		99	23
★	B-4	3.5		111	15
⊙	B-4	13.5		118	16

Document 00 73 00 - Supplementary Conditions (General Contracting - HB 96 Changes) State of Ohio Standard Requirements for Public Facility Construction

Certifications

These Supplementary Conditions amend and supplement the General Conditions and other provisions of the Contract Documents as indicated below. All provisions not amended remain in full force and effect. The terms in these Supplementary Conditions defined in the Contracting Definitions or the General Conditions shall have the meanings assigned to them in those documents.

These Supplementary Conditions are authorized, by the Ohio Facilities Construction Commission, for use on projects constructed for the [Ohio Department of Natural Resources](#).

Contracting Authority

[Ohio Department of Natural Resources](#)
2045 Morse Road E-3
Columbus, OH 43229
614-265-6948
<http://engineering.ohiodnr.gov>

Owner

[Ohio Department of Natural Resources](#)
2045 Morse Road E-3
Columbus, OH 43229
614-265-6948
<http://engineering.ohiodnr.gov>

MODIFICATIONS TO GENERAL CONDITIONS

Delete Section 1.1.3 in its entirety.

Replace 9.5.1 with:

9.5.1 Partial payments to the Contractor for labor shall be made at the rate of 96 percent of the amount invoiced through the Contractor Payment Request.

Delete Section 9.5.2 in its entirety.

Delete Section 9.7.1 in its entirety.

Replace Section 9.7.2 with the following:

9.7.2 When the Contractor has achieved Substantial Completion of all Work and there is no other reason to retain funds, the retained funds shall be paid to the Contractor within 30 days after the Contractor's request, except to the extent necessary, in the Contracting Authority's sole discretion, to assure the Contractor's faithful completion of the Work including but not limited to compliance with **Section 6.25.2**.

Insert Section 9.7.4.

9.7.4 If the funds retained under **Section 9.5.1** accrued interest while withheld from the Contractor, the payments of those retained funds to the GC will include that accrued interest.

Change Section 9.7.4 to 9.7.5

END OF DOCUMENT

1. PART 1 - GENERAL

1.1 SECTION INCLUDES

- A. The furnishing and installation of the law enforcement lockers, including all accessories as shown on drawings and specified herein.

1.2 SHOP DRAWINGS

- A. Submit shop drawings of all locker installations, showing precise locations, fabrication details and installation details, to the Architect for approval prior to starting fabrication.
- B. Submit color card.

2. PART 2 - PRODUCTS

2.1 MANUFACTURERS

- A. Lockers shall be as manufactured by Republic or equals by Superior, Penco and Debourgh.

2.2 LOCKER TYPES

- A. Provide a quantity of (5) 24"Wx36"Dx90"H law enforcement lockers in Office 116.
- B. Provide a quantity of (1) modified ADA accessible law enforcement locker in Office 116.
 - 1) Modified ADA accessible law enforcement locker shall be same dimensions and have same components as standard unit, however shelf, rod, hooks, and additional side compartments shall be below 48" A.F.F. to meet accessibility guidelines.

2.3 METAL LOCKERS

- A. Upper Unit - Prime, high-grade Class 1 mild annealed, cold-rolled A.S.T.M. A1008/A steel free from surface imperfections. Optional A.S.T.M.-A653/A Galvannealed steel is available for high humidity atmospheres. 16-gauge sides, top, and bottom. 18 gauge back.
- B. Frame - 16-gauge formed steel channels. Vertical members shall have an additional flange to form continuous door strike. Corners shall be lapped and welded into a rigid assembly. Bottom cross members shall have a tang at each end that fits through slot in rear flange of upright frame member to prevent twisting out of alignment. The top and bottom cross members shall provide support for front edge of locker top and locker bottom.

- C. Hinges – 16-gauge continuous hinges secured with rivets to door frame and to door. Hinge to have ½" knuckles and 0.235" diameter.
- D. Locker Door – One-piece, 14-gauge steel with both vertical edges formed into channel-shaped formation; top and bottom shall be flanged at 90-degree angle. The door includes a 7-1/2" wide full height 18 gauge pan reinforcement welded to inside of door with peg board area for user supplied peg board style hooks. Two groups of 6 - 3-1/2" wide louvers located at bottom of door.
- E. Handles – Upper unit door recess handle to be Nickel plated, zinc alloy with a tensile strength of not less than 40,000 psi. No moving parts are to operate against outside surface of locker. Padlock attachment to be integral part of lift which shall be attached directly to the locking bar.
- F. Quiet Locking Device – Upper door locking device shall engage frame at three points. Channel shaped locking device with full length reinforcing ribs shall be a quiet design utilizing nylon guide inserts to reduce metal to metal contact. The locking device shall include a Zinc alloy latch finger with a nano roller at point of contact with the door jamb. Lock bar shall be enclosed on three sides and operate within the channel formation of the door. Locking device shall be pre-locking so mechanism can be locked in open position with door locking automatically when closed. Security box shall have one-point locking device with a 14-gauge lock clip for attaching padlock. Doors include lock-hole to permit use of built in key or combination lock.
- G. Electrical Cutout – 1-1/4" square cutout in each rear corner of top and shelves for electrical feed.
- H. Finish: Before enamel is applied, the surfaces of the steel shall be thoroughly cleaned and phosphatized in a seven-stage process. All parts shall then be finished with a heavy coat of enamel, baked on at 300° for 30 minutes.
- I. Color: Doors and exposed body parts shall be finished in colors selected from Republic's collection of twenty-five colors. Non-exposed body parts shall be finished as well.
- J. Provide necessary trim and filler pieces as required for complete installation.

3. PART 3 - EXECUTION

- 3.1 The lockers shall be installed on bases as shown and/or specified. Lockers shall be securely anchored to base and walls, in accordance with the manufacturer's written recommendations. All anchors and fasteners shall be galvanized or cadmium plated.

3.2 ADJUSTMENT

- A. Upon completion of installation, inspect lockers and adjust as necessary for proper door and locking mechanism operation. Touch up scratches and abrasions to match original finish.

241224
Ohio Department of Natural Resources
Grand Lake St. Marys State Park – Park Office
DNR-250013

◆ END OF SECTION ◆

1. PART 1 - GENERAL

1.1 SECTION INCLUDES

- A. Requirements for automatic transfer switch for packaged engine generators.

1.2 QUALITY ASSURANCE

- A. NFPA 70 – National Electric Code
- B. NFPA 110 – Emergency and Standby Power Systems Level 1
- C. NEMA ICS 1

2. PRODUCTS 2 – PRODUCTS

2.1 GENERATOR INLET RECEPTACLE

- A. Provide a 50A inlet receptacle: Qty. (1) for emergency power rated at 50 amperes, shall be NEMA 3R, clamshell design with gaskets, 120/240v, 1Φ, corrosion resistant metallic box or non-metallic box.
- B. Must have an indicator light for a successful connection.
- C. Inlet receptacle shall be tested and labeled by the Underwriters Laboratories.

2.2 GENERATOR CONNECTION CORD

- A. Provide a replacement cord for the existing generator.
- B. Must be a minimum of 20', with and be exterior rated, SOOW cable.
- C. Cord must have a 50A plug on both ends, NEMA 14-50P. Contractor to verify with existing equipment prior to work.

2.3 GENERATOR TRANSFER SWITCH PANEL

- A. Provide a manual transfer switch: Qty. (1) for emergency power rated at 50 amperes, 3 pole, 3 wire 120/240 volt – 60Hz mounted in a NEMA 1 enclosure. Include five-year comprehensive warranty.
- B. Panel to have a capacity of (10) 20A branch circuit breakers minimum.
- C. Transfer switch panel shall be tested and labeled by the Underwriters Laboratories.

3. PART 3 – EXECUTION

3.1 INSTALLATION

241224

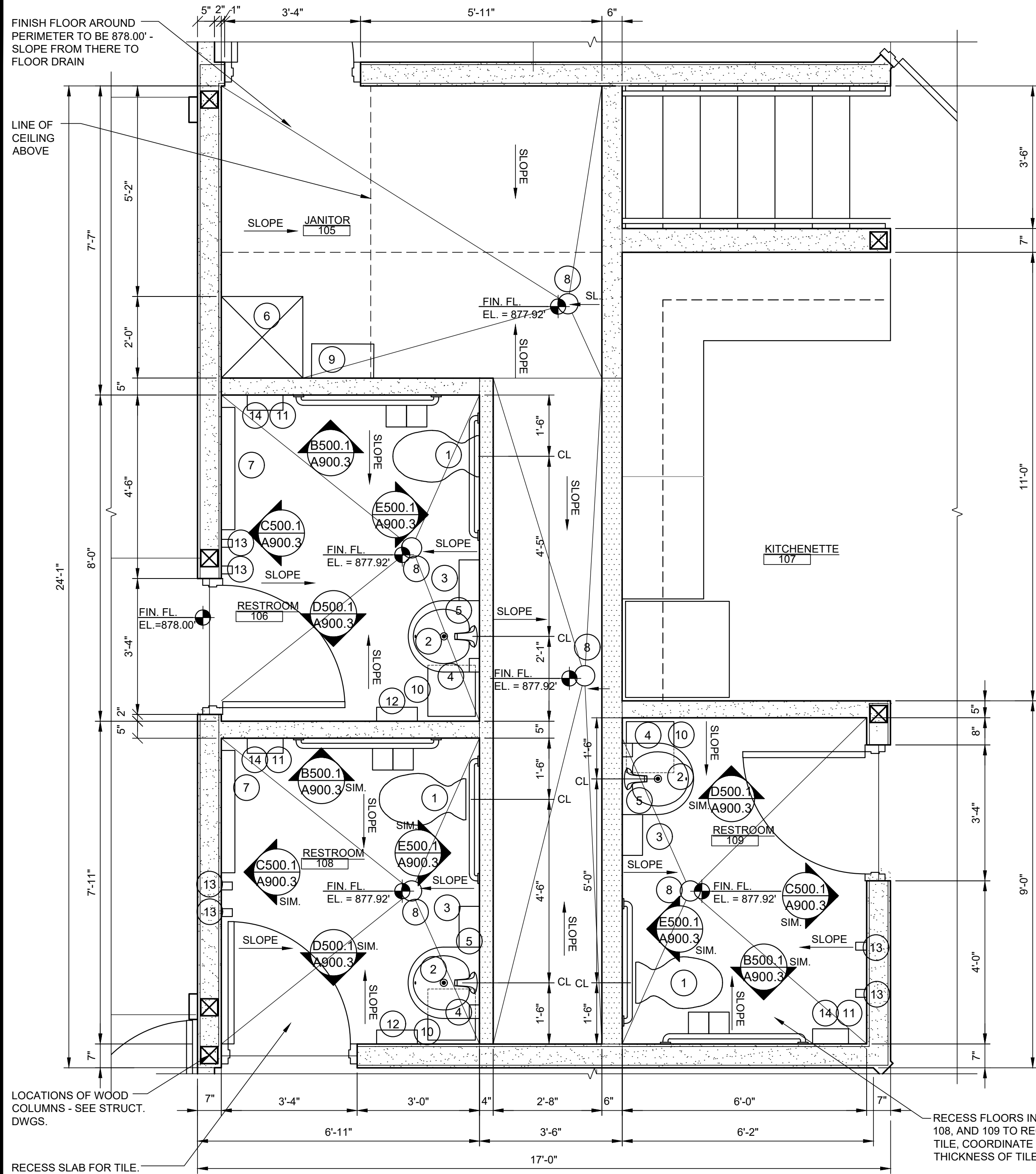
Ohio Department of Natural Resources

Grand Lake St. Marys State Park – Park Office

DNR-250013

- A. Install manual transfer switch in accordance with manufacturer's recommendations.

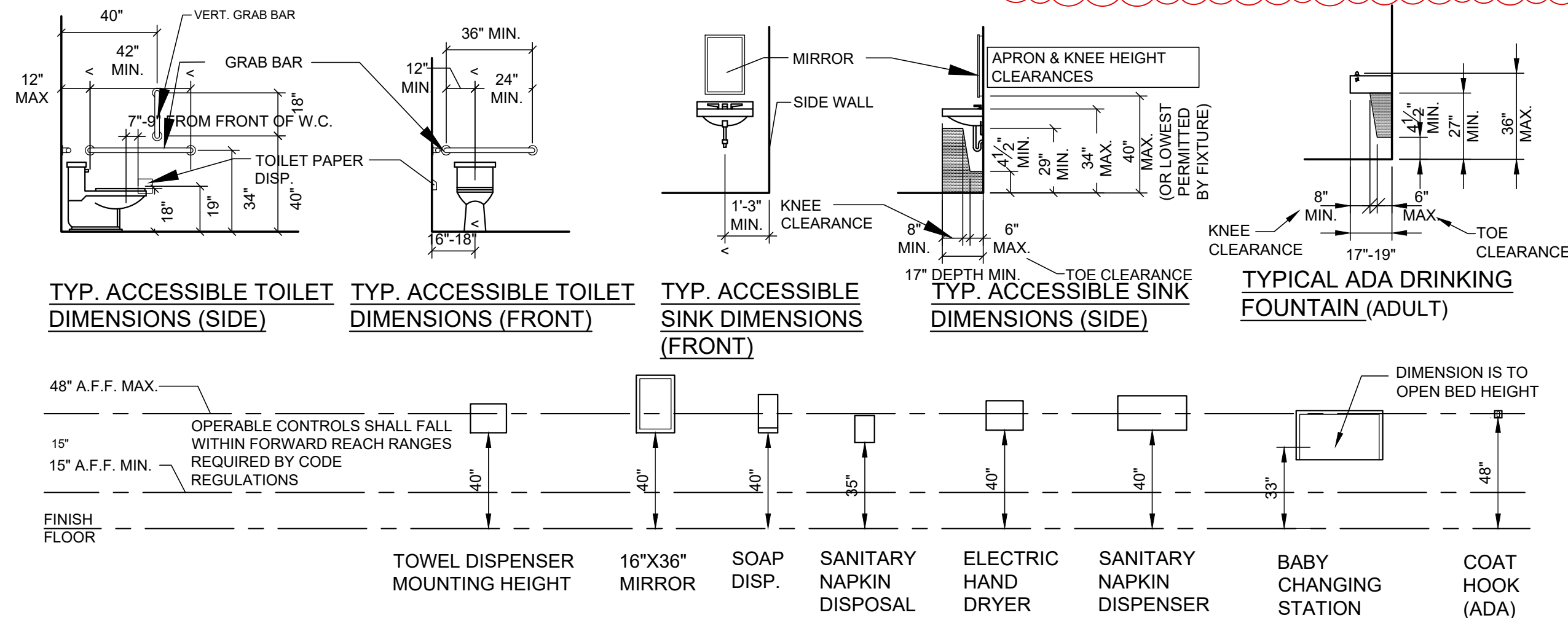
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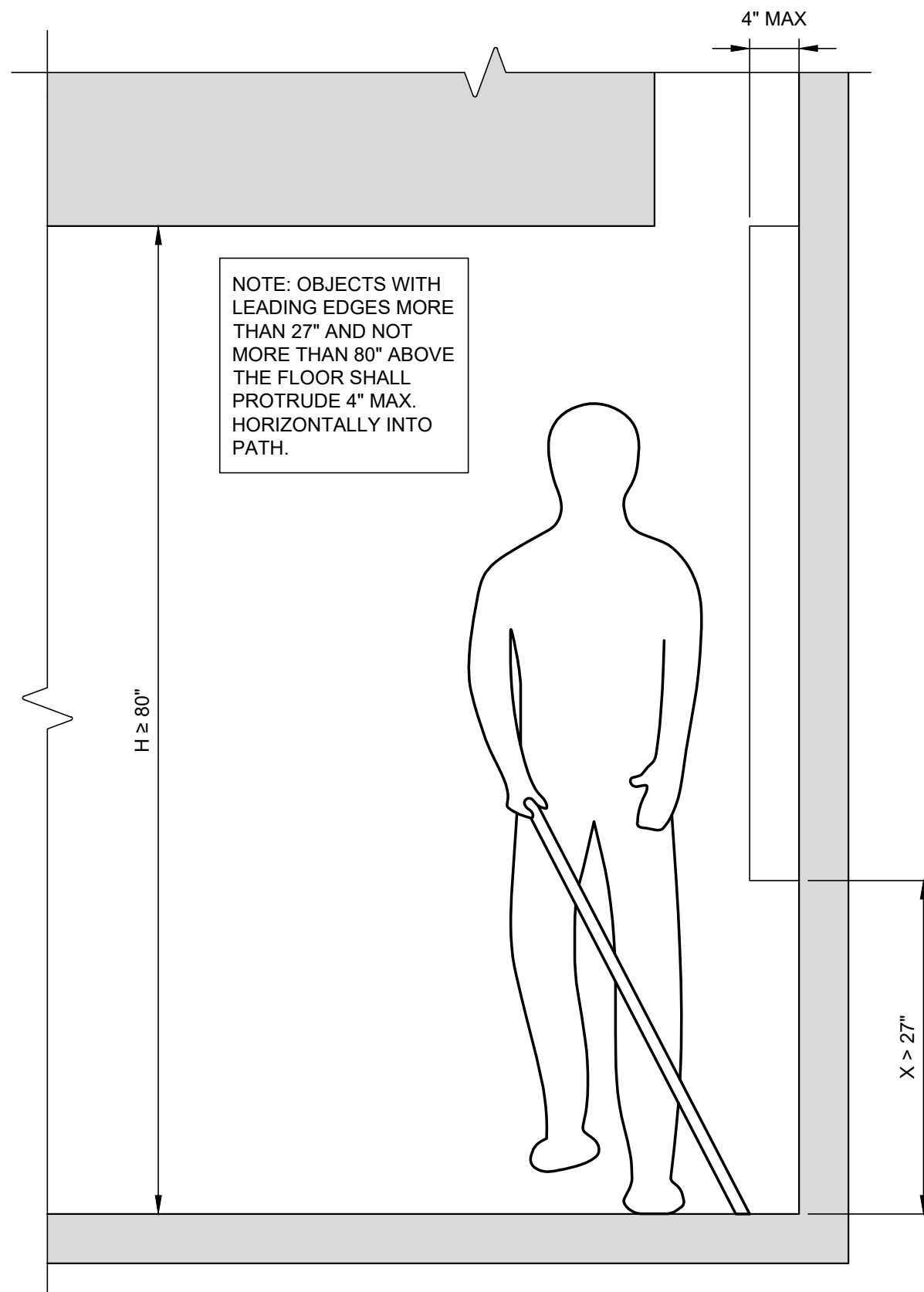
A100.1 ENLARGED RESTROOM PLAN
A500.1 SCALE: 1 1/2" = 1'-0"

- KEYED NOTES**
- 1 ACCESSIBLE TOILET WITH ASSOCIATED STAINLESS STEEL GRAB BARS AND TOILET PAPER DISPENSER. SEE PLUMBING DRAWINGS FOR MORE INFORMATION. TOILET PAPER DISPENSERS PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR.
 - 2 ACCESSIBLE WALL MOUNTED SINK. SEE PLUMBING DRAWINGS FOR MORE INFORMATION.
 - 3 PAPER TOWEL DISPENSER - PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR
 - 4 SOAP DISPENSER - PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR
 - 5 18"x36" MIRROR GLASS TO COMPLY WITH OBC SECTION 2406.1 THROUGH 2406.1.4.
 - 6 MOP SINK WITH MOP HOLDER
 - 7 BABY CHANGING STATION
 - 8 FLOOR DRAIN
 - 9 INSTANT WATER HEATER
 - 10 WASTE RECEPTACLE - PROVIDED AND INSTALLED BY OWNER
 - 11 SANITARY NAPKIN DISPENSER - PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR
 - 12 ELECTRIC HAND DRYERS
 - 13 COAT HOOKS
 - 14 SANITARY NAPKIN DISPOSAL - PROVIDED BY OWNER AND INSTALLED BY CONTRACTOR

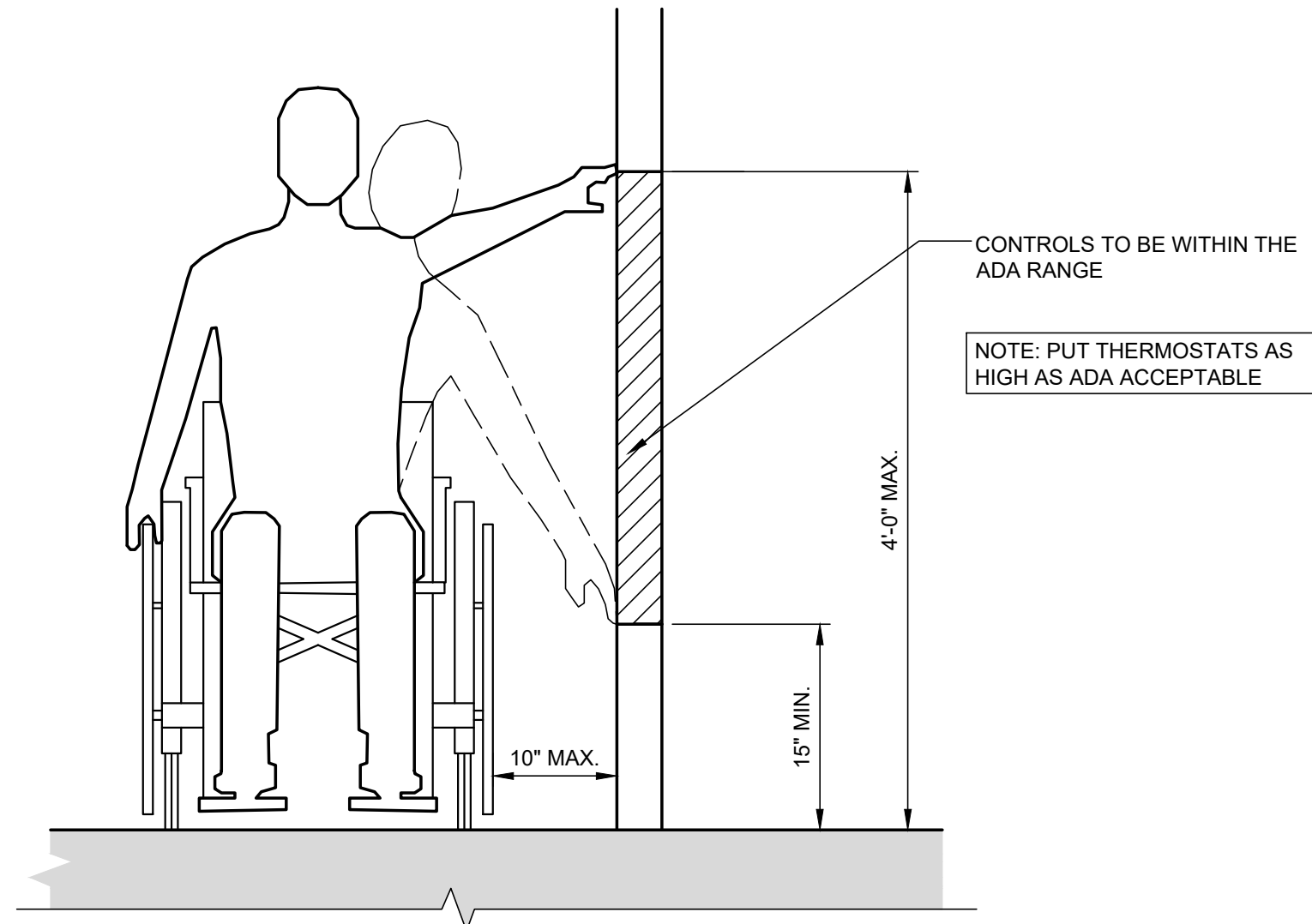
TYPICAL ACCESSIBLE MOUNTING HEIGHTS AND DIMENSIONS



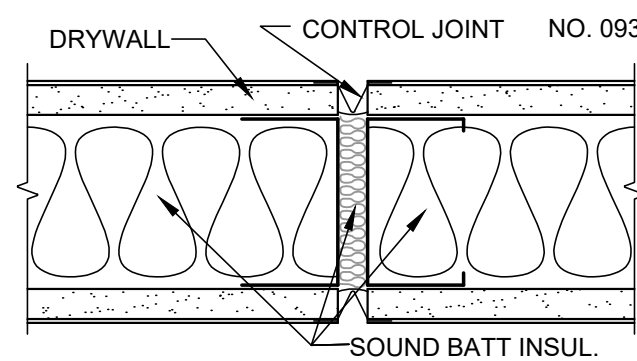
NOTE: THE PURPOSE OF THIS SECTION IS TO SHOW TYPICAL ACCESSIBLE (ICC A117.1) DIMENSIONS AND MOUNTING HEIGHTS ONLY. ALL FIXTURES AND ACCESSORIES SHOWN IN THIS SECTION ARE SHOWN GENERICALLY AND ARE FOR REFERENCE PURPOSES ONLY.



B500.1 ADA CLEARANCE
A500.1 SCALE: N.T.S.



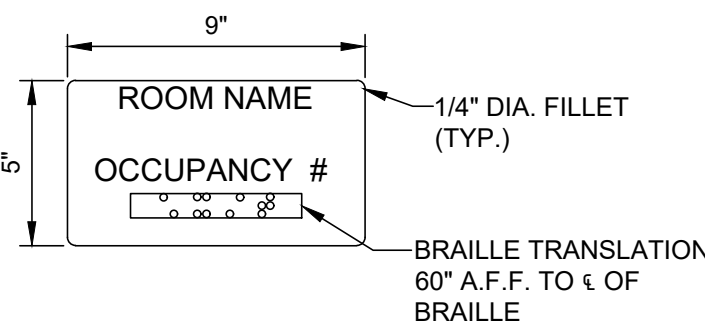
C500.1 ADA CLEARANCE
A500.1 SCALE: N.T.S.



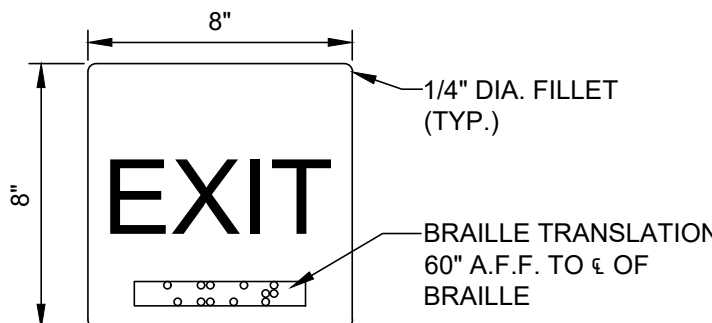
DRYWALL CJ DETAIL
SCALE: 3" = 1'-0"

RESTROOM NOTES

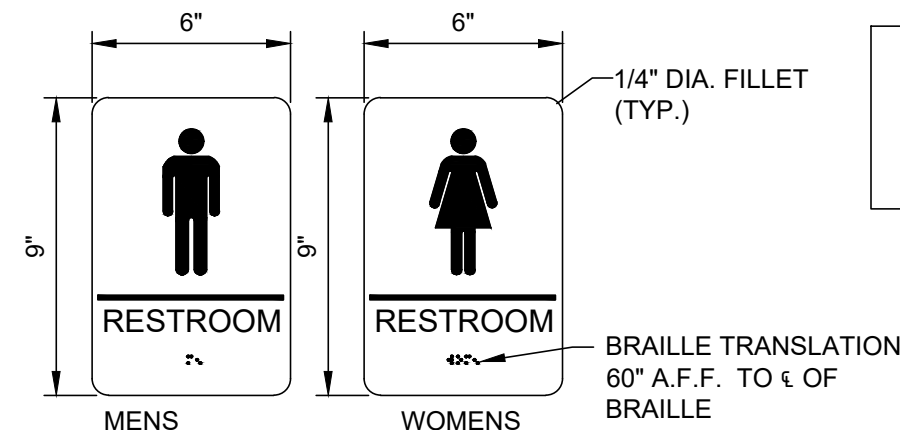
1. WATER SUPPLY AND DRAINPIES UNDER LAVATORIES AND SINKS SHALL BE INSULATED OR OTHERWISE CONFIGURED TO PROTECT AGAINST CONTACT IN ADA UNIT. THERE SHALL BE NO SHARP OR ABRASIVE SURFACES UNDER LAVATORIES AND SINKS.
2. SLOPE ALL RESTROOM FLOORS TOWARDS DRAINS A MAXIMUM OF 1/8" PER FOOT.
3. BEFORE FINAL FLOOR FINISH IS INSTALLED IN DRAINED AREAS OF RESTROOMS, FLOORS SHALL BE WATER TESTED / FLOODED TO INSURE NO PUDDLING AND POSITIVE DRAINAGE OCCURS BEFORE FINAL FINISH COAT IS APPLIED.



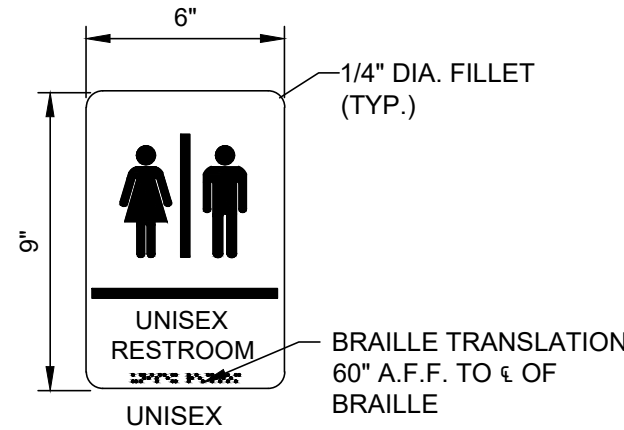
D500.1 OCCUPANCY SIGN DETAIL
A500.1 SCALE: 2"=1'-0"



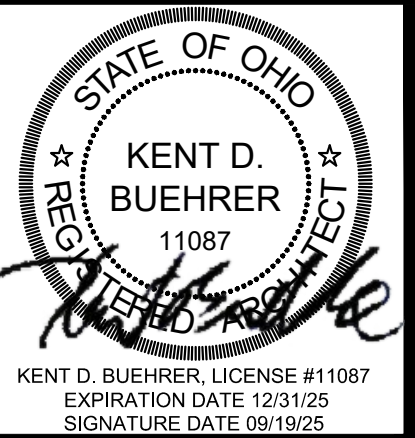
E500.1 TACTILE EXIT SIGN DETAIL
A500.1 SCALE: 2"=1'-0"



F500.1 TACTILE RESTROOM SIGN DETAIL
A500.1 SCALE: 2"=1'-0"



G500.1 TACTILE UNISEX SIGN DETAIL
A500.1 SCALE: 2"=1'-0"

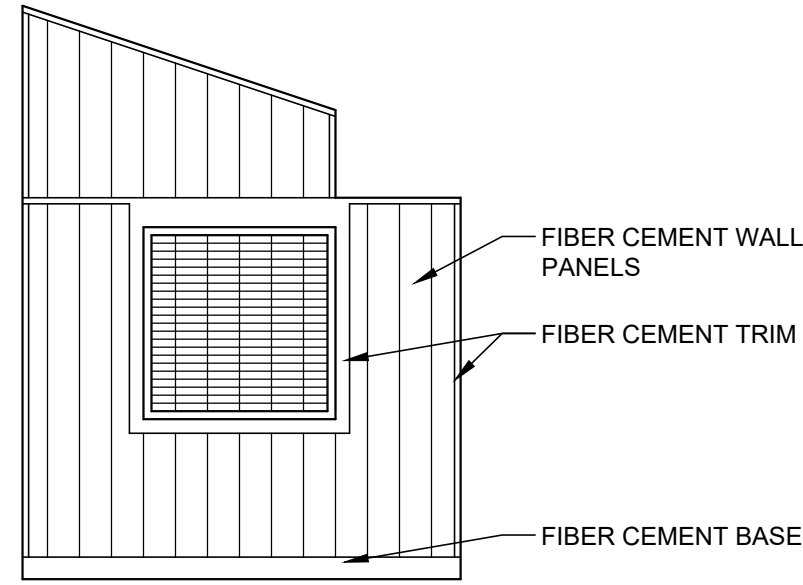


GLSM PARK OFFICE
FOR
OHIO DEPARTMENT OF NATURAL
RESOURCES DNR-250013
AUGLAZE COUNTY, OHIO

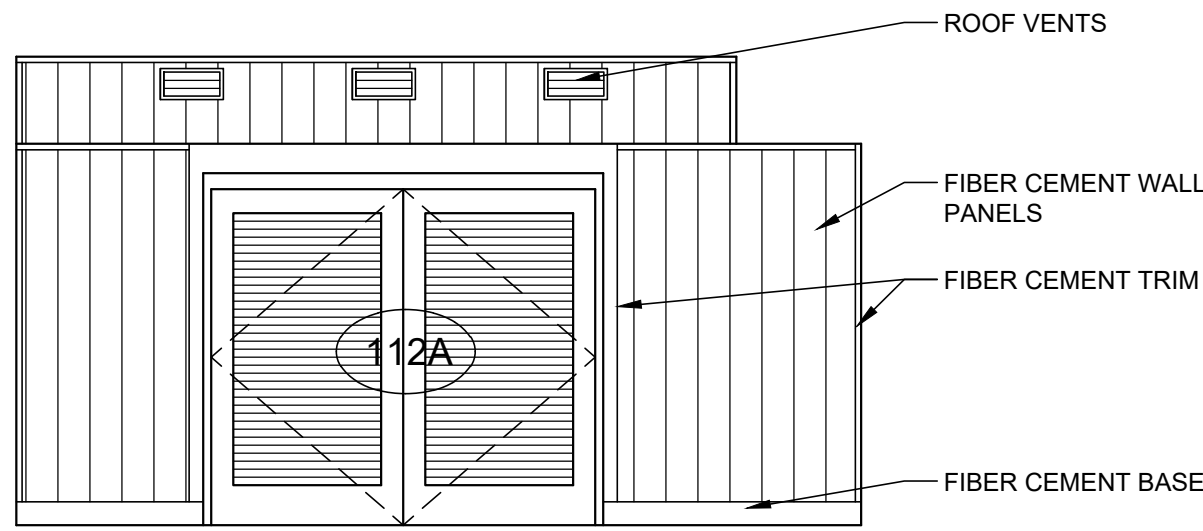
BUEHRERGROUP
architecture & engineering, inc.
314 CONANT ST., WAUMEE, OH 43337 | P: 419.893.9021 | F: 419.893.9027 | BUEHRERGROUP.COM

REVISION	SCHEDULE
1	ISSUE DATE: 09/19/2025
2	JOB NUMBER: 241224
3	CONSTRUCTION DOCUMENTS

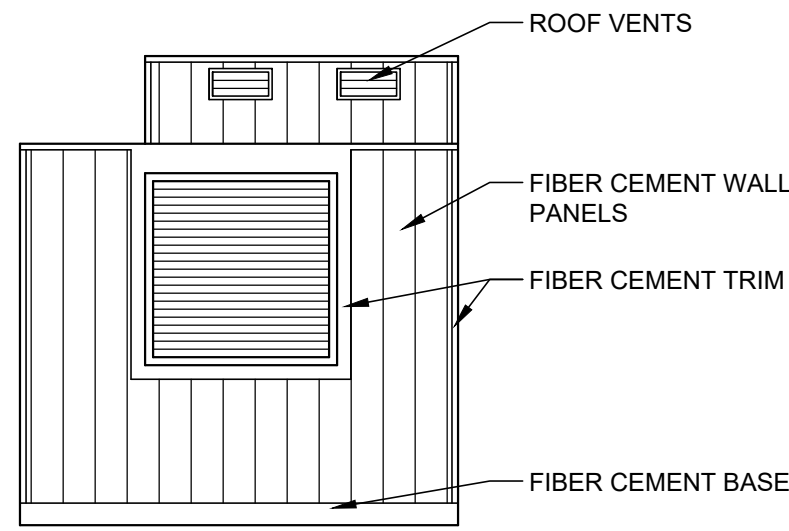
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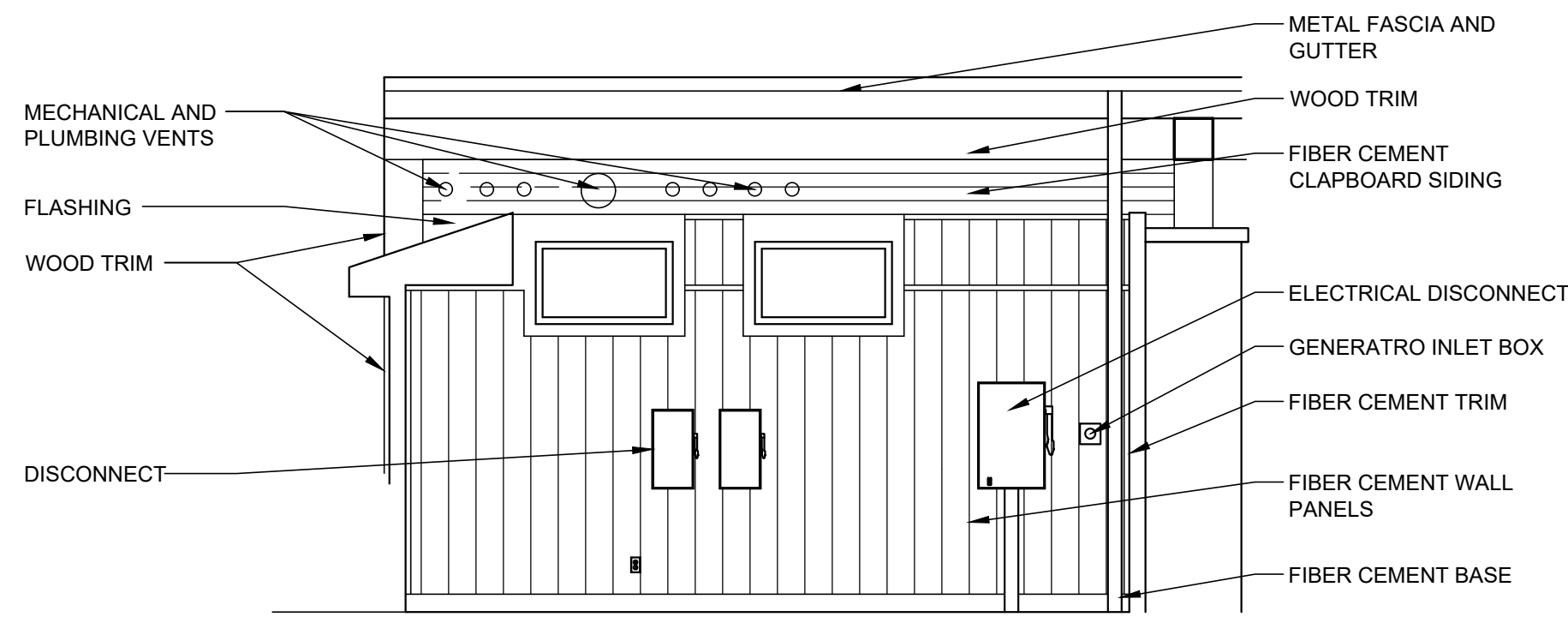
MECHANICAL
YARD ELEVATION
A200.1
A900.4
SCALE: 1/4"=1'-0"



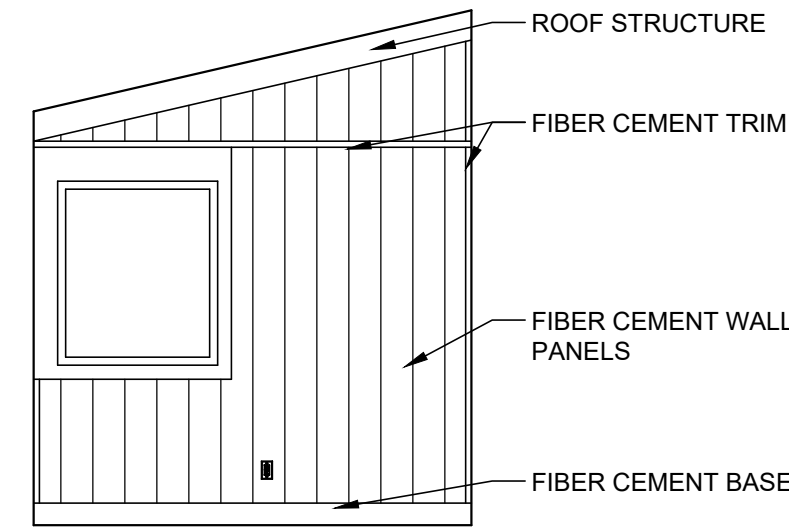
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YARD ELEVATION
B200.1
A900.4
SCALE: 1/4"=1'-0"



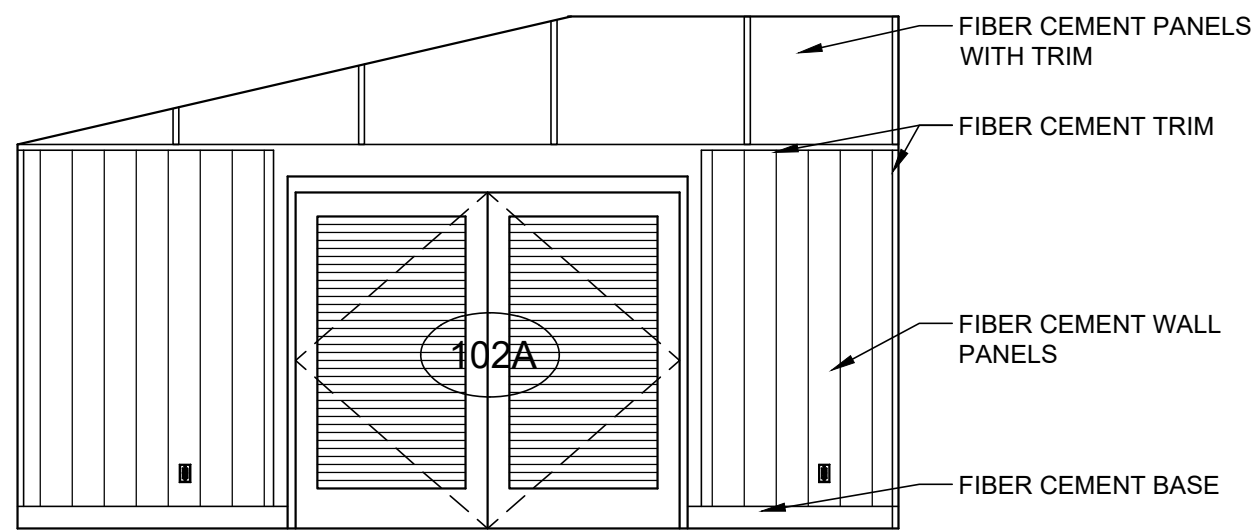
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YARD ELEVATION
C200.1
A900.4
SCALE: 1/4"=1'-0"



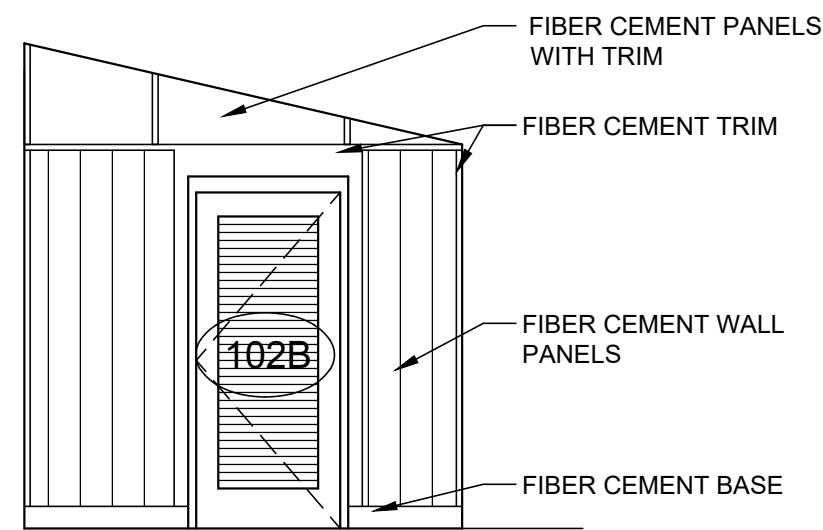
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YARD ELEVATION
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A900.4
SCALE: 1/4"=1'-0"



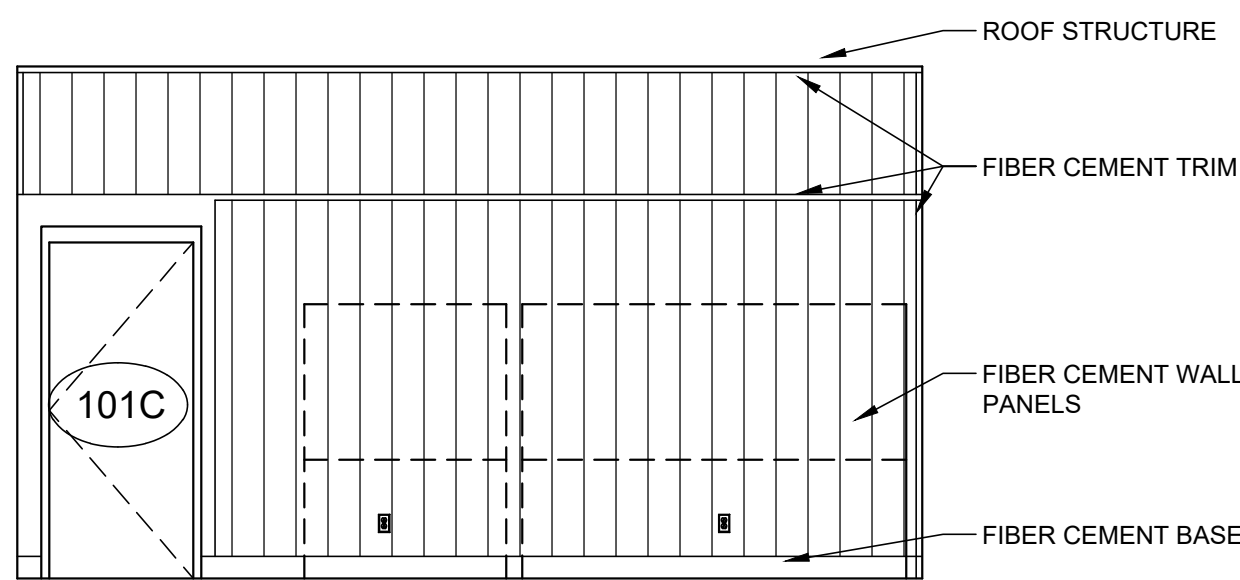
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STORAGE ELEVATION
E200.1
A900.4
SCALE: 1/4"=1'-0"



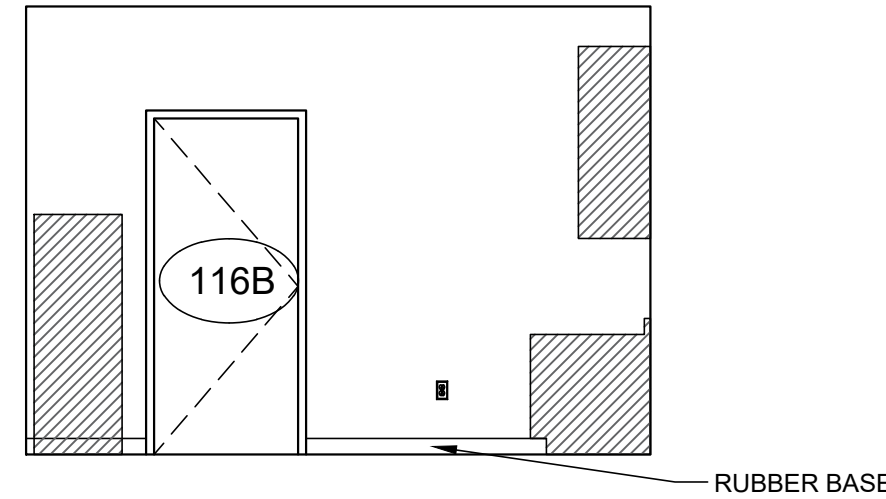
EXTERIOR
STORAGE ELEVATION
F200.1
A900.4
SCALE: 1/4"=1'-0"



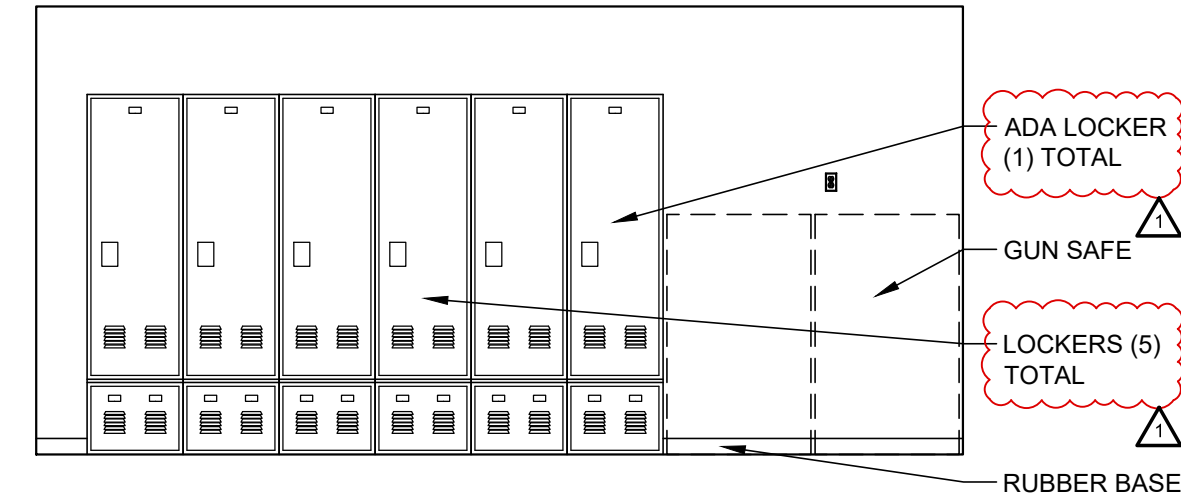
EXTERIOR
STORAGE ELEVATION
G200.1
A900.4
SCALE: 1/4"=1'-0"



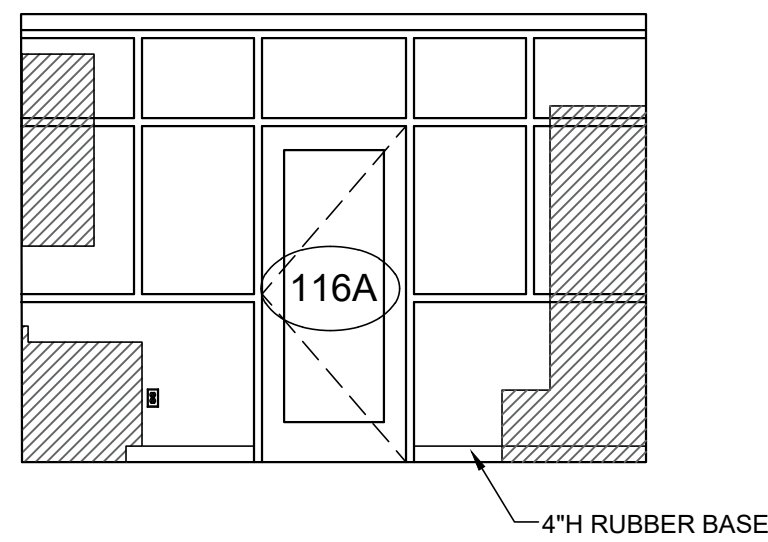
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STORAGE ELEVATION
H200.1
A900.4
SCALE: 1/4"=1'-0"



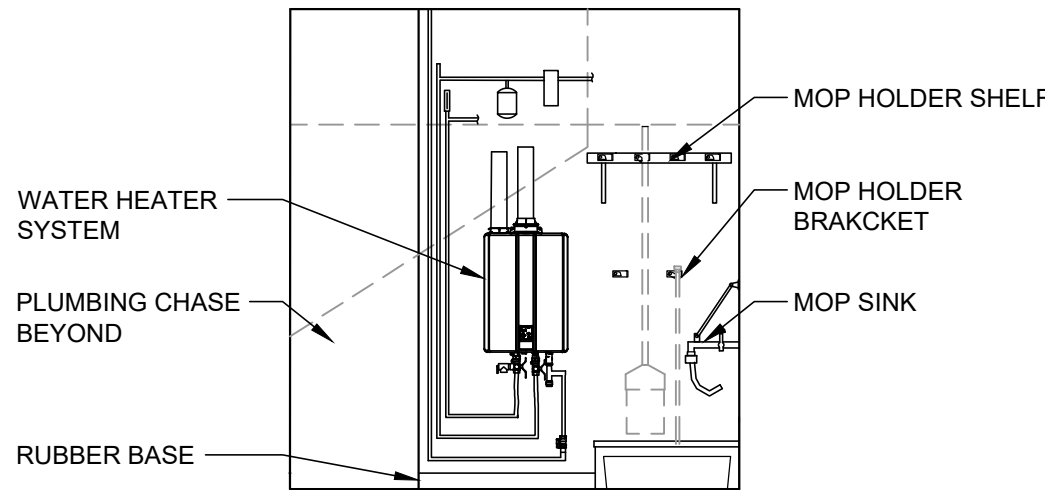
OFFICE 116
J200.1
A900.4
SCALE: 1/4"=1'-0"



OFFICE 116
K200.1
A900.4
SCALE: 1/4"=1'-0"



OFFICE 116
L200.1
A900.4
SCALE: 1/4"=1'-0"



JANITOR 105
M200.1
A900.4
SCALE: 1/4"=1'-0"

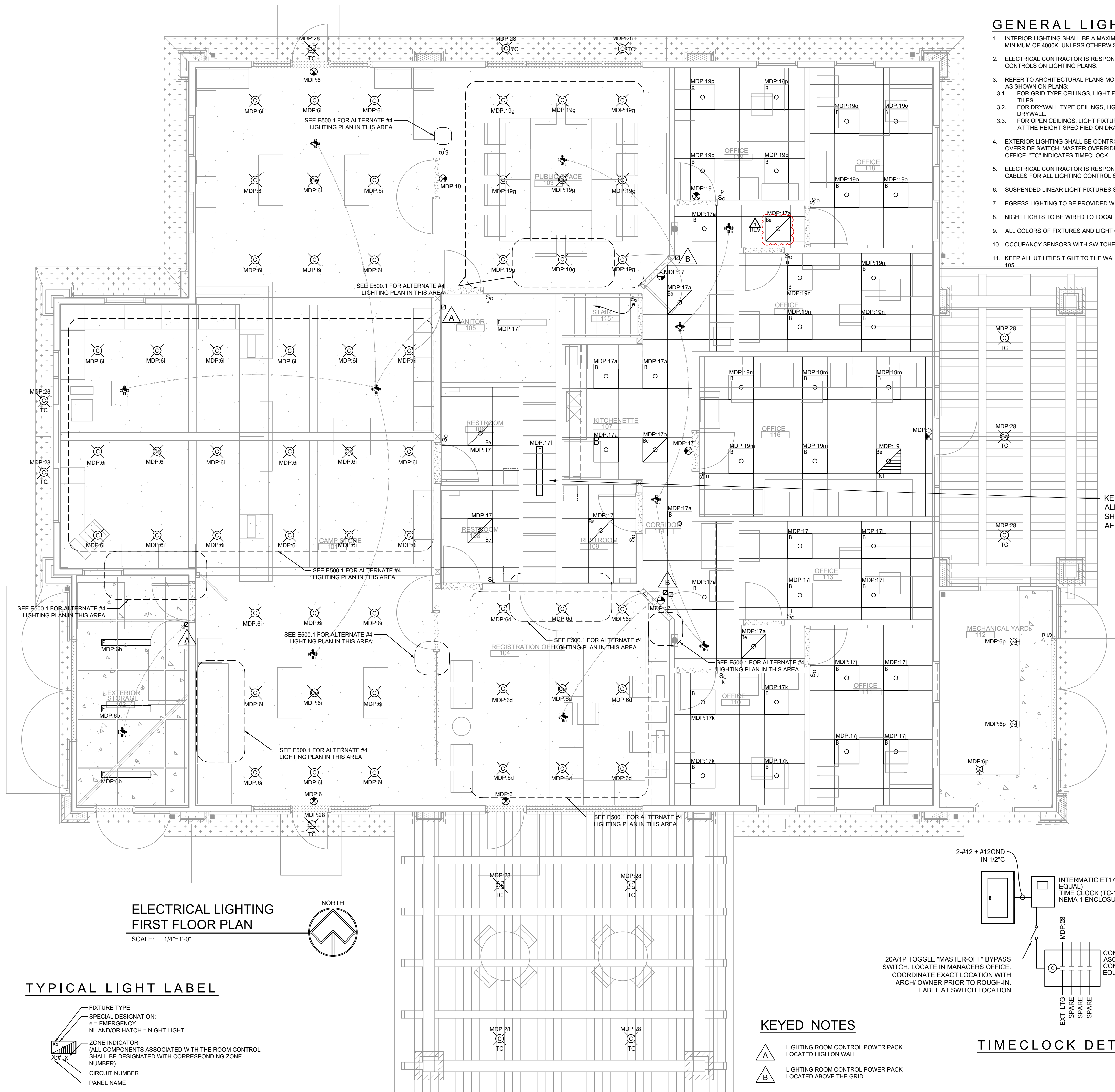


GLSM PARK OFFICE
FOR
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AUGLAZE COUNTY,
OHIO



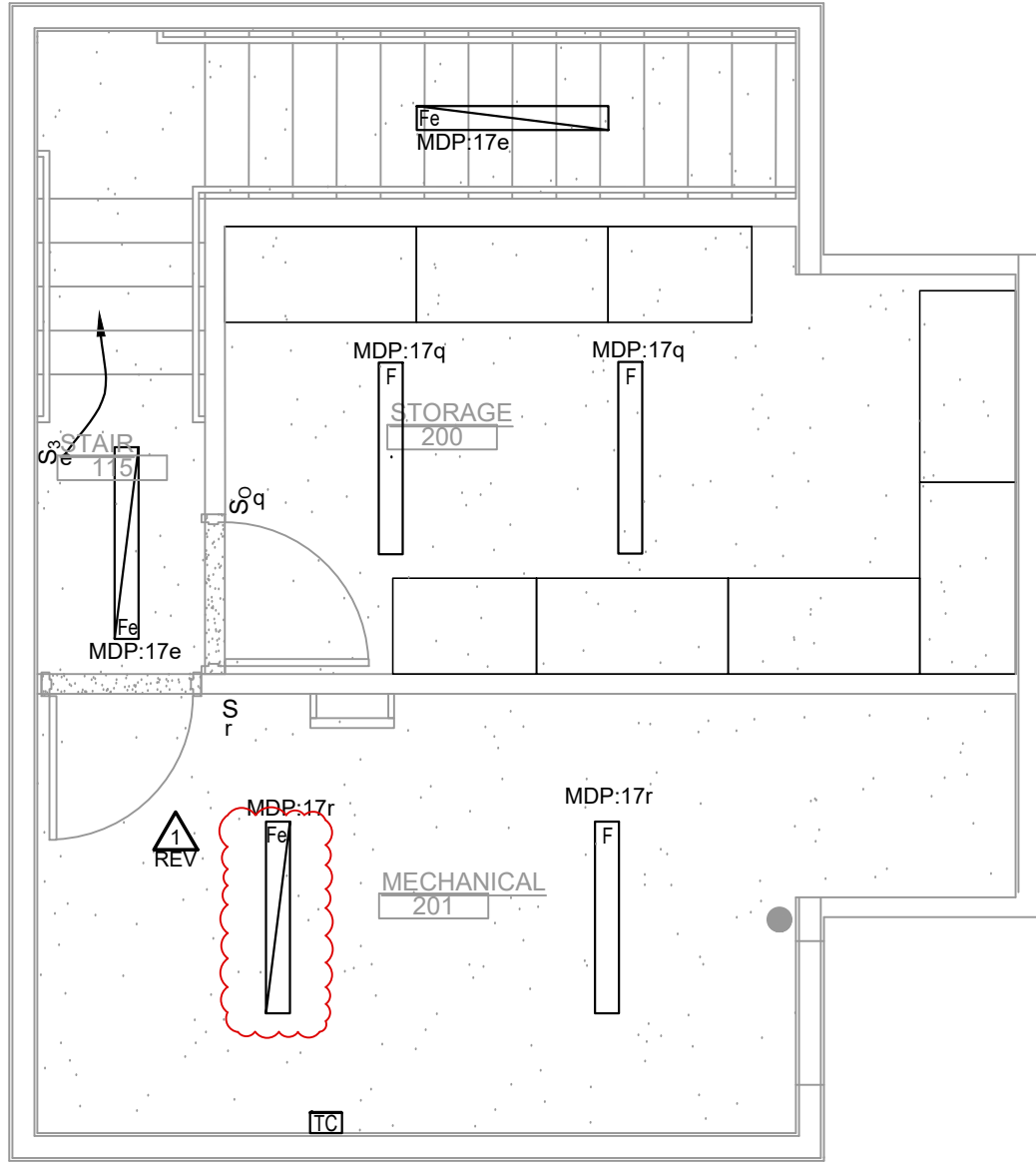
REVISION SCHEDULE	
Δ	
ISSUE DATE: 09/19/2025	
JOB NUMBER: 241224	
CONSTRUCTION DOCUMENTS	

A900.4



GENERAL LIGHTING NOTES

- INTERIOR LIGHTING SHALL BE A MAXIMUM OF 3500K, EXTERIOR LIGHTING SHALL BE A MINIMUM OF 4000K, UNLESS OTHERWISE NOTED.
- ELECTRICAL CONTRACTOR IS RESPONSIBLE FOR REFERRING TO THE LIGHTING CONTROLS ON LIGHTING PLANS.
- REFER TO ARCHITECTURAL PLANS MOUNTING HEIGHTS FOR LIGHTING FIXTURES ARE AS SHOWN ON PLANS:
 - FOR GRID TYPE CEILINGS, LIGHT FIXTURES ARE TO BE MOUNTED INSIDE GRID TILES.
 - FOR DRYWALL TYPE CEILINGS, LIGHT FIXTURES ARE TO BE MOUNTED IN DRYWALL.
 - FOR OPEN CEILINGS, LIGHT FIXTURES SHALL BE MOUNTED VIA AIRCRAFT CABLE AT THE HEIGHT SPECIFIED ON DRAWINGS.
- EXTERIOR LIGHTING SHALL BE CONTROLLED WITH A TIMECLOCK & A MASTER OVERRIDE SWITCH. MASTER OVERRIDE SWITCH TO BE LOCATED IN MANAGERS OFFICE. "TC" INDICATES TIMECLOCK.
- ELECTRICAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL LOW-VOLTAGE CABLES FOR ALL LIGHTING CONTROL SYSTEM COMPONENTS.
- SUSPENDED LINEAR LIGHT FIXTURES SHALL BE LEVEL AND PARALLEL TO FLOOR.
- EGRESS LIGHTING TO BE PROVIDED WITH 90 MINUTE BATTERY BACK-UP.
- NIGHT LIGHTS TO BE WIRED TO LOCAL ROOM LIGHTING CIRCUIT OF SWITCH.
- ALL COLORS OF FIXTURES AND LIGHT CONTROLS TO BE SELECTED BY ARCHITECT.
- OCCUPANCY SENSORS WITH SWITCHES TO BE PROVIDED FOR ALL LOCATIONS.
- KEEP ALL UTILITIES TIGHT TO THE WALL IN THE PLUMBING CHASE OF JANITOR ROOM 105.



ELECTRICAL LIGHTING MEZZANINE PLAN

SCALE: 1/4"=1'-0"

LIGHT FIXTURE LEGENDS

- | | | | |
|----------------|---|---|---|
| B | LED FIXTURE 1-30W-24"-LED-120V RECESSED 2' x 2' DIMMABLE (0-10V) LED FLAT PANEL TROFFER - 3500K - FOR GRID TYPE CEILING. EQUIP WITH LED DRIVER. "w" INDICATES BATTERY BACKUP. HATCHED SYMBOL DENOTES A NIGHTLIGHT, WIRED AHEAD OF THE SWITCH. | LITHONIA
METALUX
HUBBELL
OR APPROVED EQUAL | CPX 2x2 3400LM 80CRI 35K MIN10 ZT MVOLT
22FP235C
CFP22-3335 |
| C | LED FIXTURE 1-15W-6"-LED-120V RECESSED 6" LED DOWNLIGHT - MATTE WHITE - 1500 LUMENS - 3500K. NEW CONSTRUCTION HOUSING. WET LOCATION RATED. DIMMING OPTION IN ROOM 103 IF ALTERNATE 34 IS NOT ACCEPTED. | LITHONIA
PRESOLITE
HALO | LDN6 35/15 LOWWR MVOLT
LF6SL-6LFSL1535K8WT
PD615ED10-PDM6A835-61VWB |
| F | LED FIXTURE 1-40W-48"-LED-120V 2' x 4' LED STRIPLIGHT - "X" -3500K | METALUX
LITHONIA
CURRENT | 4SLSTP4040DD-UNV
ZL1D L48 5000LM FST MVOLT 35K 80CRI WH
LC14-35M-EU |
| H | OUTDOOR LED - 1-18W-LED-120V, WALL MOUNTED LED WALL PACK- 4000K SEE ARCHITECTURAL ELEVATIONS FOR EXACT LOCATION. COLOR TO BE SELECTED BY ARCHITECT. | HALO
LITHONIA
ENVISION | WP-18-50-LED/WPFLD-TRN
ESKF1 AL0 SIW2 KY DDB
LED-ARL-15W-TRI-B2-TR |
| K | LED EXIT SIGN WITH 6" RED LETTERS IN WHITE PLASTIC HOUSING. MOUNTING AND ARROWS AS SHOWN. PROVIDE 90 MINUTES BATTERY BACKUP. DIFFUSE LENS. CENTER ABOVE DOORS. | LITHONIA
SURE-LITES
DUAL-LITE
CHLORIDE
COMPASS CE | LOM S W 3 R 120/277
APX7R
EVEURW
VXL3RW
CER |
| S | LOCAL SWITCH - 1 POLE - 20A - 125/277V WITH COVER PLATE. MH=4'-0". | HUBBELL
P & S
COOPER
LEVITON | 1221x
CSB20AC1-x
1221x
1221-2x |
| S ₃ | LOCAL SWITCH - 3 WAY - 20A - 125/277V WITH COVER PLATE. MH=4'-0". | HUBBELL
P & S
COOPER
LEVITON | 1223x
CSB20AC3-x
1223x
1223-2x |
| S ₄ | LOCAL SWITCH - 4 WAY - 20A - 125/277V WITH COVER PLATE. MH=4'-0". | HUBBELL
P & S
COOPER
LEVITON | 1224x
CSB20AC4-x
1224x
1224-2x |
| S ₀ | DUAL TECHNOLOGY OCCUPANCY SENSOR WALL SWITCH - 100W - 120/277V WITH COVER PLATE. MH=4'-0" | HUBBELL
P & S
WATT STOPPER
SENSORSWITCH | AD2000x1
WDT100x
DW1-100-x
WSD PDT x |
| S ₀ | LOCAL DIMMER SWITCH - LED DRIVER DIMMING (0-10V). VERIFY COMPATIBILITY WITH LIGHT FIXTURES AND POWER PACKS PRIOR TO ORDERING. | GREENGATE
LUTRON
SENSOR SWITCH
P & S
CRESTRON | WBSD-010SLD-X
NTR3TV-DV-XX
SPDMMRD X
CD4FBL3PX
CLW-DIMFLVEX-277-P-X-S |
| + | DUAL TECHNOLOGY PIR / ULTRASONIC OCCUPANCY SENSOR RECESSED IN CEILING - 24VDC. SEE SPECS. COVERS UP TO 1,000 FT. ² WITH 360° FIELD OF VIEW. EQUIP WITH LOW-VOLTAGE POWER PACK. | GREENGATE
SENSOR SWITCH
WATTSTOPPER | OAC-DT-1000
RM PDT 9
DT-300 |
| □ | OCCUPANCY SENSOR CONTROL UNIT / POWER PACK - 120/277V - MOUNT ABOVE CEILING IN NEMA 1 ENCLOSURE. SEE SPECS. | GREENGATE
SENSOR SWITCH
WATTSTOPPER | SP20-MV
MP20
BZ-50 |

ELECTRICAL LIGHTING FIRST FLOOR PLAN

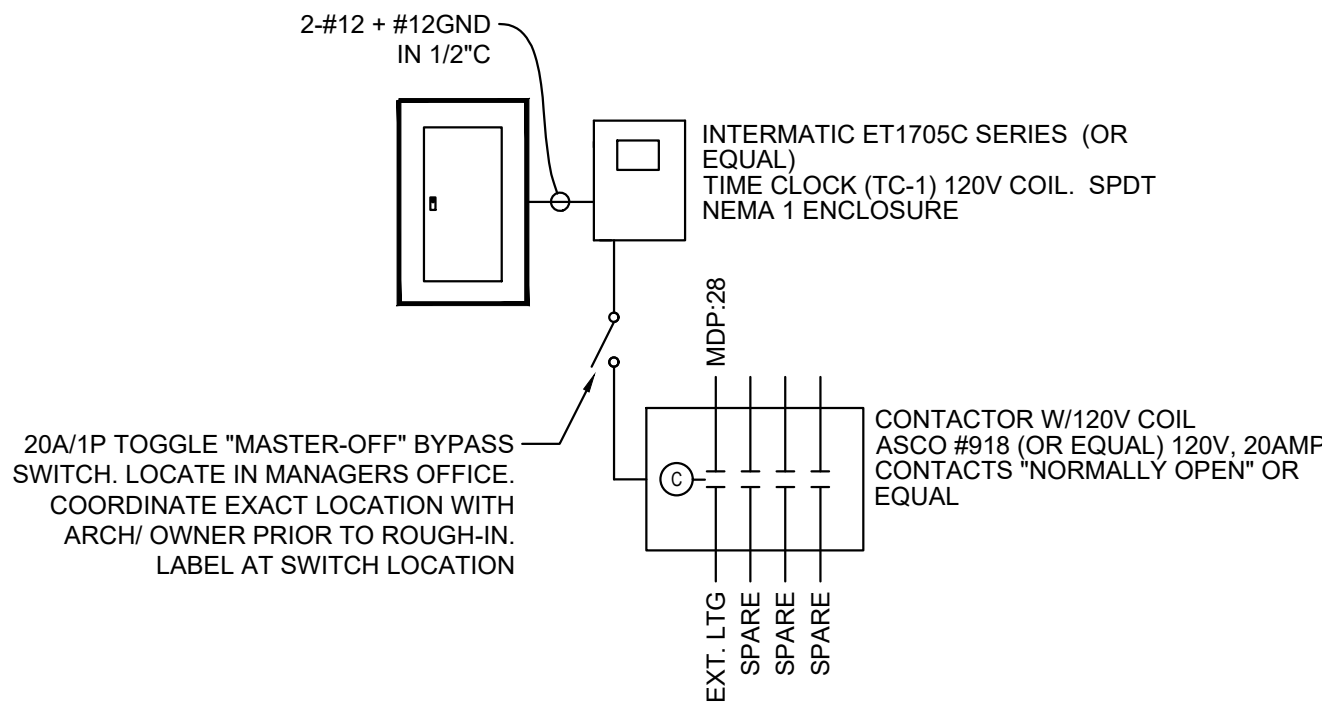
SCALE: 1/4"=1'-0"

TYPICAL LIGHT LABEL

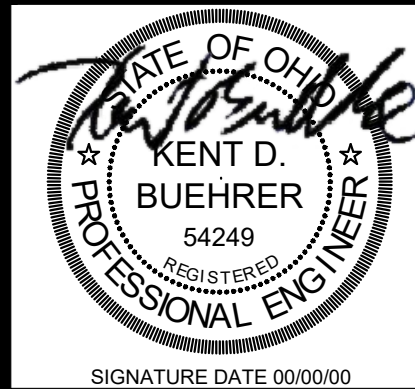
- FIXTURE TYPE
- SPECIAL DESIGNATION:
 - e = EMERGENCY
 - NL AND/OR HATCH = NIGHT LIGHT
- ZONE INDICATOR (ALL COMPONENTS ASSOCIATED WITH THE ROOM CONTROL SHALL BE DESIGNATED WITH CORRESPONDING ZONE NUMBER)
- CIRCUIT NUMBER
- PANEL NAME

KEYED NOTES

- A LIGHTING ROOM CONTROL POWER PACK LOCATED HIGH ON WALL.
- B LIGHTING ROOM CONTROL POWER PACK LOCATED ABOVE THE GRID.



TIMECLOCK DETAIL

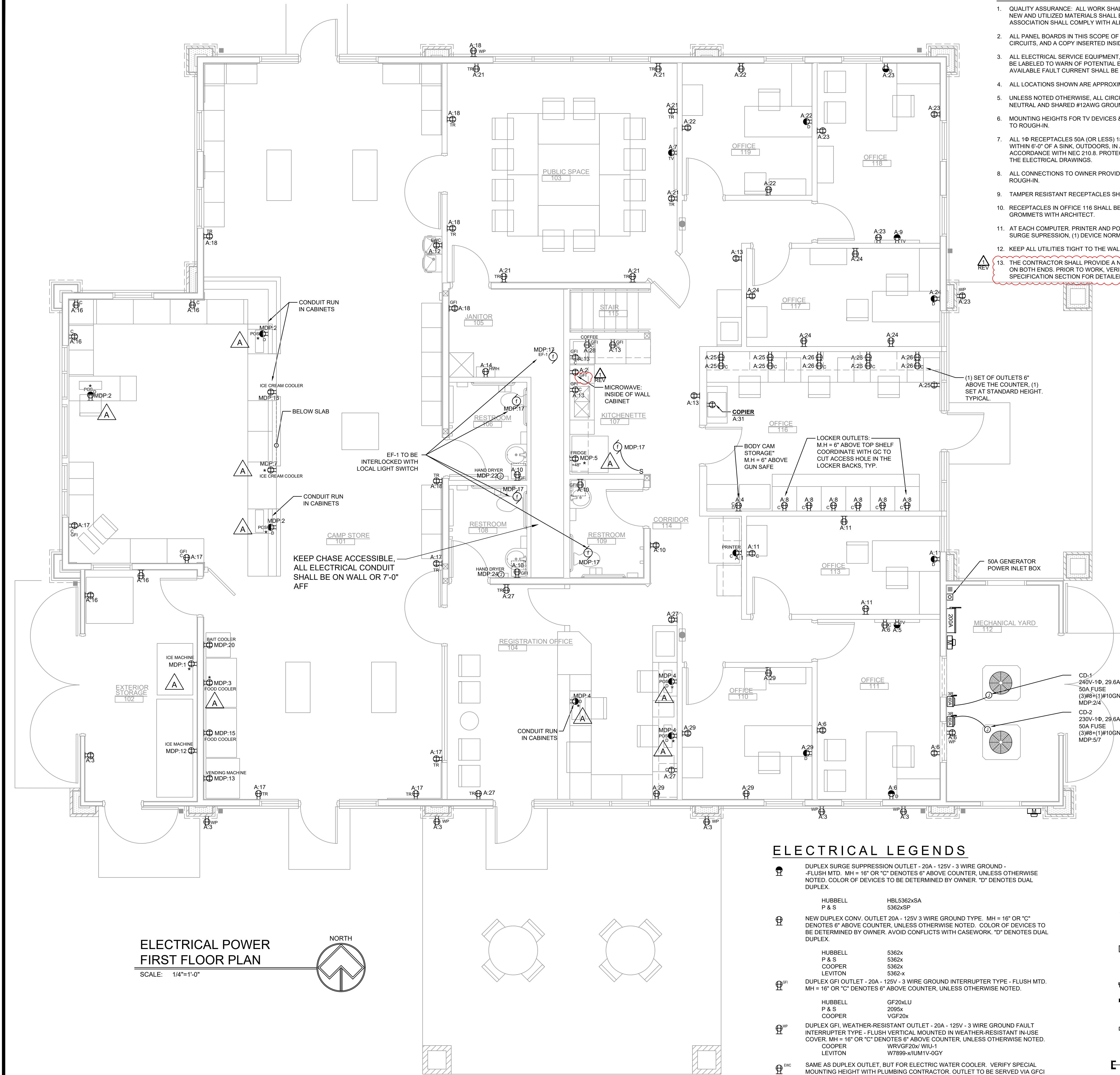


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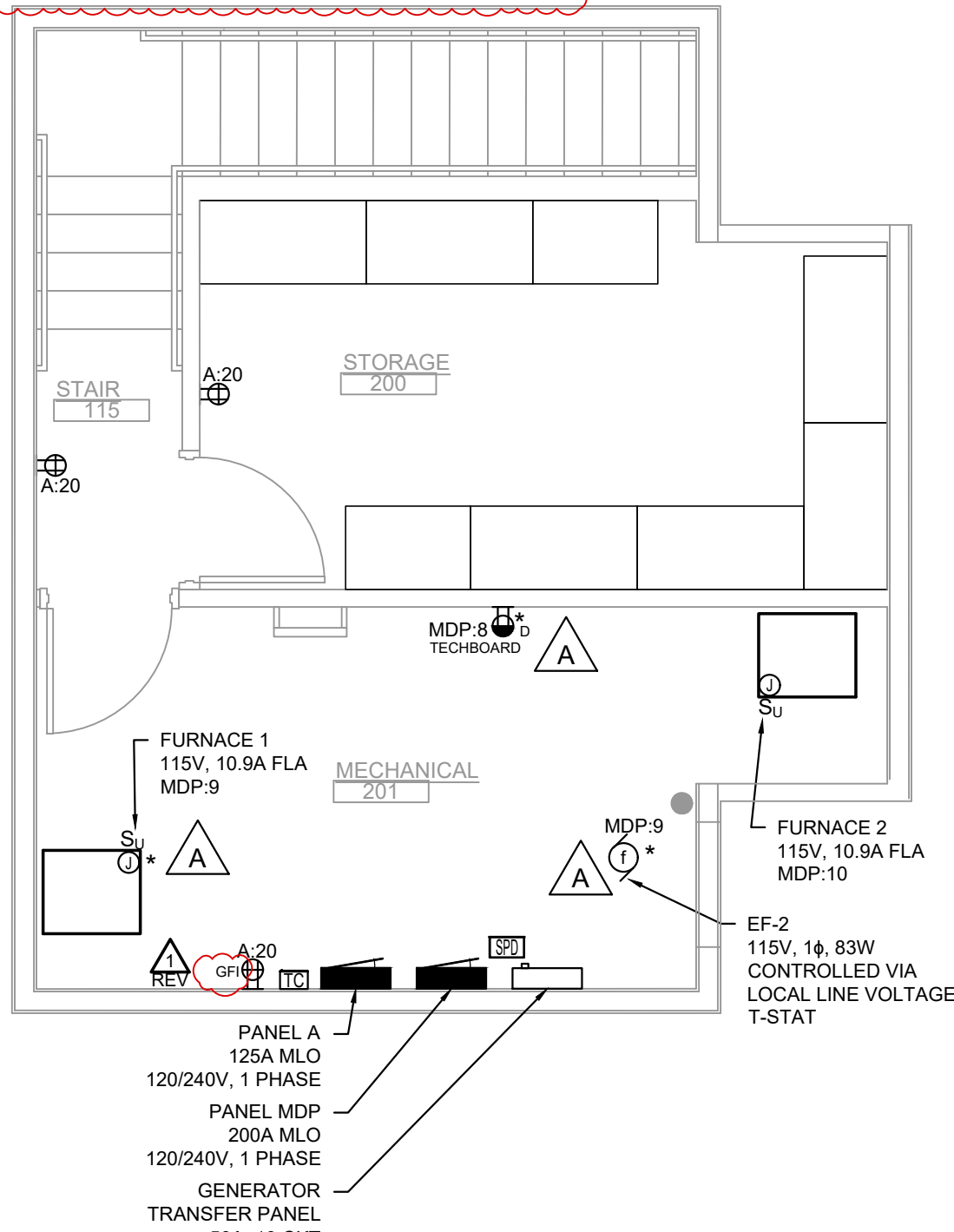
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CONSTRUCTION DOCUMENTS

E100.1



GENERAL POWER NOTES

1. QUALITY ASSURANCE: ALL WORK SHALL BE IN ACCORDANCE WITH THE NFPA 70 - NATIONAL ELECTRICAL CODE 2023. ALL NEW AND UTILIZED MATERIALS SHALL BE UL - UNDERWRITER'S LABORATORY LISTED. OCCUPATIONAL HEALTH AND SAFETY ASSOCIATION SHALL COMPLY WITH ALL APPLICABLE OSHA STANDARDS.
2. ALL PANEL BOARDS IN THIS SCOPE OF WORK SHALL ALL HAVE LABELED TYPED DIRECTORIES PROVIDED, INDICATING ALL CIRCUITS, AND A COPY INSERTED INSIDE THE PANEL DOOR.
3. ALL ELECTRICAL SERVICE EQUIPMENT, PANELBOARDS, AND ANY ADDITIONAL EQUIPMENT DETERMINED BY THE AHJ, SHALL BE LABELED TO WARN OF POTENTIAL ELECTRICAL ARC HAZARDS, IN ACCORDANCE WITH NEC 110.16. ADDITIONALLY, THE AVAILABLE FAULT CURRENT SHALL BE INDICATED AT THE SERVICE GEAR IN ACCORDANCE WITH NEC 110.24.
4. ALL LOCATIONS SHOWN ARE APPROXIMATE.
5. UNLESS NOTED OTHERWISE, ALL CIRCUIT HOMERUNS ON THIS SHEET SHALL BE #12AWG WITH DEDICATED #12AWG NEUTRAL AND SHARED #12AWG GROUND WIRE, IN 3/4" CONDUIT.
6. MOUNTING HEIGHTS FOR TV DEVICES & ASSOCIATED DEVICES TO BE COORDINATED WITH TV MOUNTING BRACKETS PRIOR TO ROUGH-IN.
7. ALL 1Ø RECEPTACLES 50A (OR LESS) 150V TO GND (OR LESS), OR 3Ø RECEPTACLES 100A (OR LESS) 150V TO GND (OR LESS) WITHIN 6'-0" OF A SINK, OUTDOORS, IN A KITCHEN, BATHROOM, OR ON A ROOFTOP SHALL BE GFCI PROTECTED - TYPICAL, IN ACCORDANCE WITH NEC 210.8. PROTECTION MAY BE INTEGRAL TO THE DEVICE OR AT THE PANELBOARD, AS INDICATED BY THE ELECTRICAL DRAWINGS.
8. ALL CONNECTIONS TO OWNER PROVIDED EQUIPMENT SHALL BE FIELD VERIFIED FOR ACTUAL LOCATION AND SIZE PRIOR TO ROUGH-IN.
9. TAMPER RESISTANT RECEPTACLES SHALL BE PROVIDED IN ALL AREAS LISTED IN NEC ARTICLE 406.12.
10. RECEPTACLES IN OFFICE 116 SHALL BE LOCATED BELOW AND ABOVE COUNTER. COORDINATE FINAL LOCATION OF GROMMETS WITH ARCHITECT.
11. AT EACH COMPUTER, PRINTER AND POS LOCATION WHERE THERE IS A DOUBLE DUPLEX SHOWN, (1) DEVICE SHALL BE SURGE SUPPRESSION, (1) DEVICE NORMAL OPERATION
12. KEEP ALL UTILITIES TIGHT TO THE WALL IN THE PLUMBING CHASE OF JANITOR ROOM 105.
13. THE CONTRACTOR SHALL PROVIDE A NEW 20-FOOT GENERATOR CONNECTION CORD EQUIPPED WITH A NEMA 14-50P PLUG ON BOTH ENDS. PRIOR TO WORK, VERIFY COMPATIBILITY WITH EXISTING EQUIPMENT. REFER TO THE APPLICABLE SPECIFICATION SECTION FOR DETAILED REQUIREMENTS.



ELECTRICAL POWER MEZZANINE PLAN

SCALE: 1/4"=1'-0"

KEYED NOTES

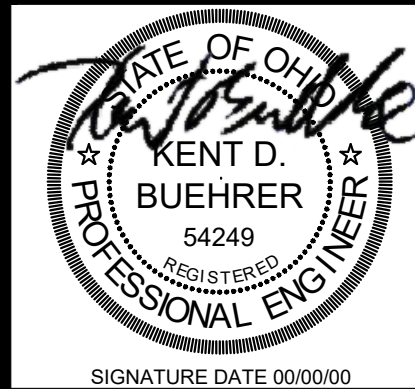
△ CIRCUITS CONNECTED TO THE GENERATOR PANEL

ELECTRICAL LEGENDS

- | | | | |
|--|--|--|---|
| | DUPLEX SURGE SUPPRESSION OUTLET - 20A - 125V - 3 WIRE GROUND - FLUSH MTD. MH = 16" OR "C" DENOTES 6" ABOVE COUNTER, UNLESS OTHERWISE NOTED. COLOR OF DEVICES TO BE DETERMINED BY OWNER. "D" DENOTES DUAL DUPLEX. | | DUPLEX TAMPER-RESISTANT OUTLET - 20A - 125V - 3 WIRE GROUND TYPE - FLUSH MTD. MH = 16" OR "C" DENOTES 6" ABOVE COUNTER, UNLESS OTHERWISE NOTED. |
| | HUBBELL P & S | | HUBBELL BR20xTR |
| | HUBBELL 5362xSA | | HUBBELL TR20x |
| | HUBBELL 5362xSP | | HUBBELL TRBR20x |
| | NEW DUPLEX CONV. OUTLET 20A - 125V 3 WIRE GROUND TYPE. MH = 16" OR "C" DENOTES 6" ABOVE COUNTER, UNLESS OTHERWISE NOTED. COLOR OF DEVICES TO BE DETERMINED BY OWNER. AVOID CONFLICTS WITH CASEWORK. "D" DENOTES DUAL DUPLEX. | | INTERMATIC ET1705C |
| | HUBBELL 5362x | | SURGE SUPPRESSOR - MOUNT WITHIN 2' - 0" OF MAIN DISTRIBUTION PANEL. SEE SPECS. |
| | HUBBELL 5362x | | FRACTIONAL HORSEPOWER MOTOR BY M.C. |
| | COOPER 5362x | | SAFETY SWITCH - SIZE AS NOTED ("3R" RAIN TIGHT, "NF" NON-FUSED, "4X" STAINLESS STEEL) |
| | LEVITON 5362-x | | SURFACE MOUNTED POWER PANEL - 120/240V-1Ø-3W. |
| | DUPLEX GFI OUTLET - 20A - 125V - 3 WIRE GROUND INTERRUPTER TYPE - FLUSH MTD. MH = 16" OR "C" DENOTES 6" ABOVE COUNTER, UNLESS OTHERWISE NOTED. | | ELECTRIC METER BY UTILITY COMPANY |
| | HUBBELL GF20xLU | | GENERATOR TRANSFER SWITCH |
| | HUBBELL 2095x | | GENERATOR POWER INLET BOX |
| | HUBBELL VGF20x | | CONDUIT CAP |
| | DUPLEX GFI WEATHER-RESISTANT OUTLET - 20A - 125V - 3 WIRE GROUND FAULT INTERRUPTER TYPE - FLUSH VERTICAL MOUNTED IN WEATHER-RESISTANT IN-USE COVER. MH = 16" OR "C" DENOTES 6" ABOVE COUNTER, UNLESS OTHERWISE NOTED. | | JUNCTION BOX |
| | COOPER WRVGF20x | | |
| | LEVITON W7899-x/IUM1V-0GY | | |
| | SAME AS DUPLEX OUTLET, BUT FOR ELECTRIC WATER COOLER. VERIFY SPECIAL MOUNTING HEIGHT WITH PLUMBING CONTRACTOR. OUTLET TO BE SERVED VIA GFCI BREAKER. | | |
| | SAME AS DOUBLE DUPLEX OUTLET BUT FOR T.V. COORDINATE MOUNTING HEIGHT WITH BRACKET LOCATION. | | |

ELECTRICAL POWER FIRST FLOOR PLAN

SCALE: 1/4"=1'-0"



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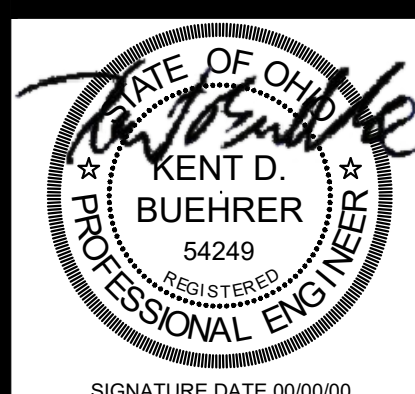
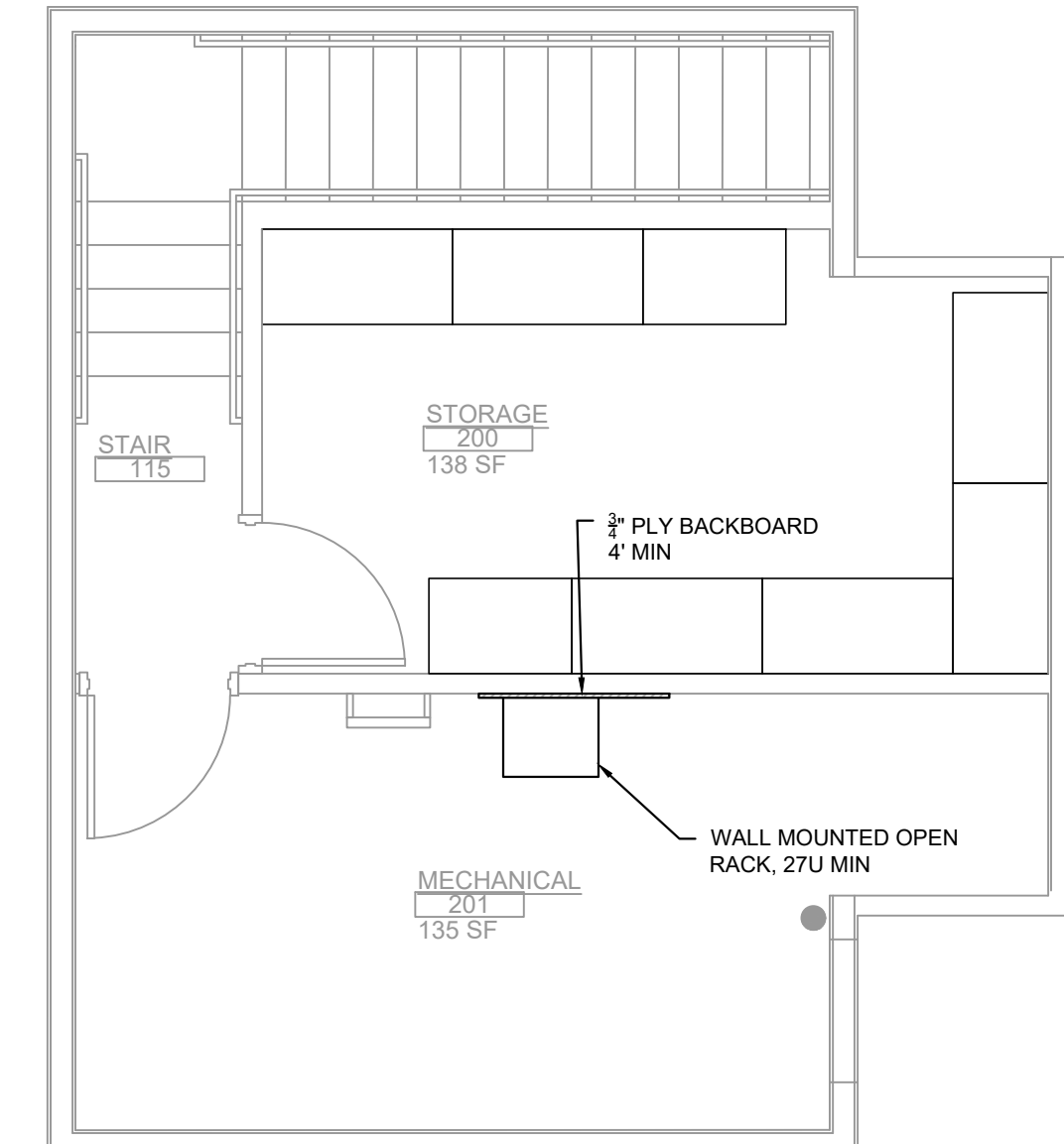


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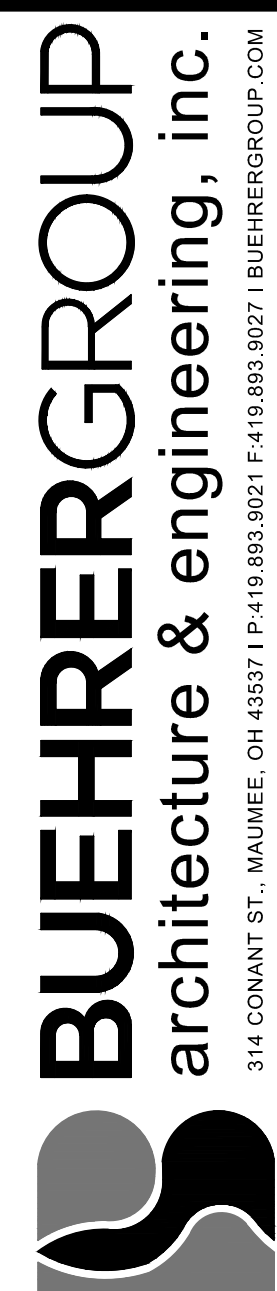
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CONSTRUCTION DOCUMENTS

E200.1



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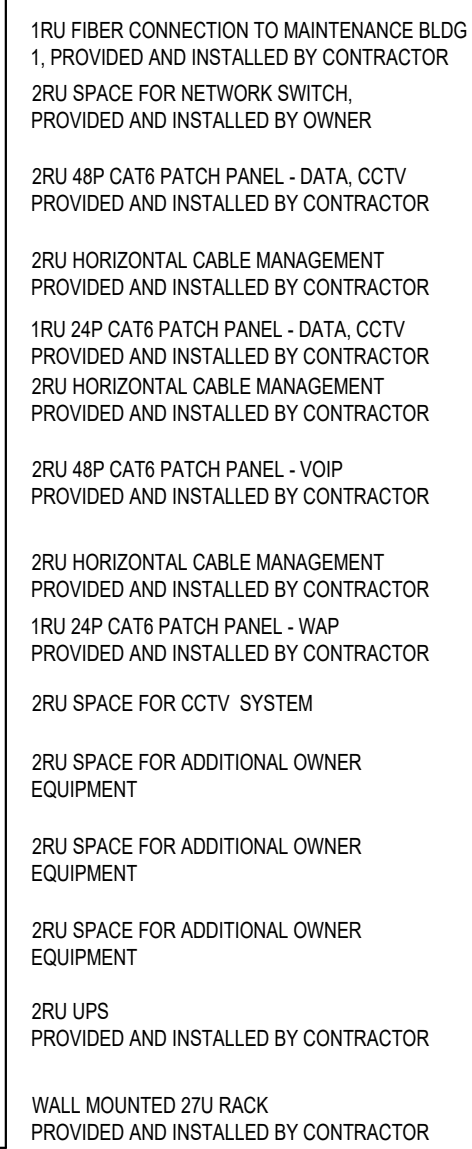


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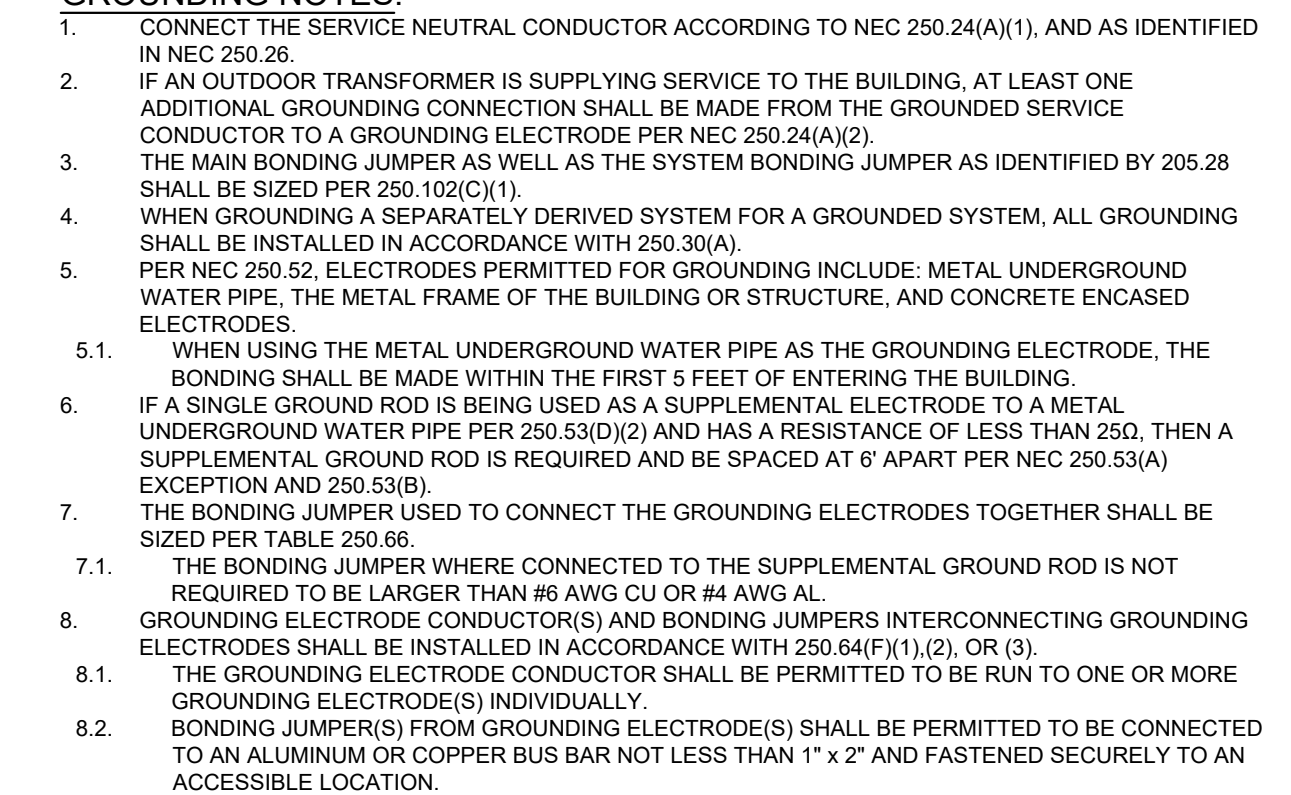
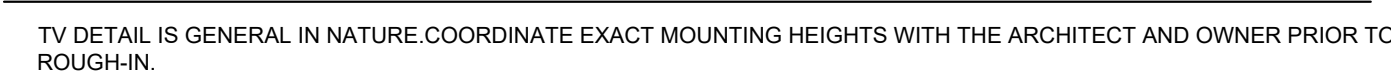
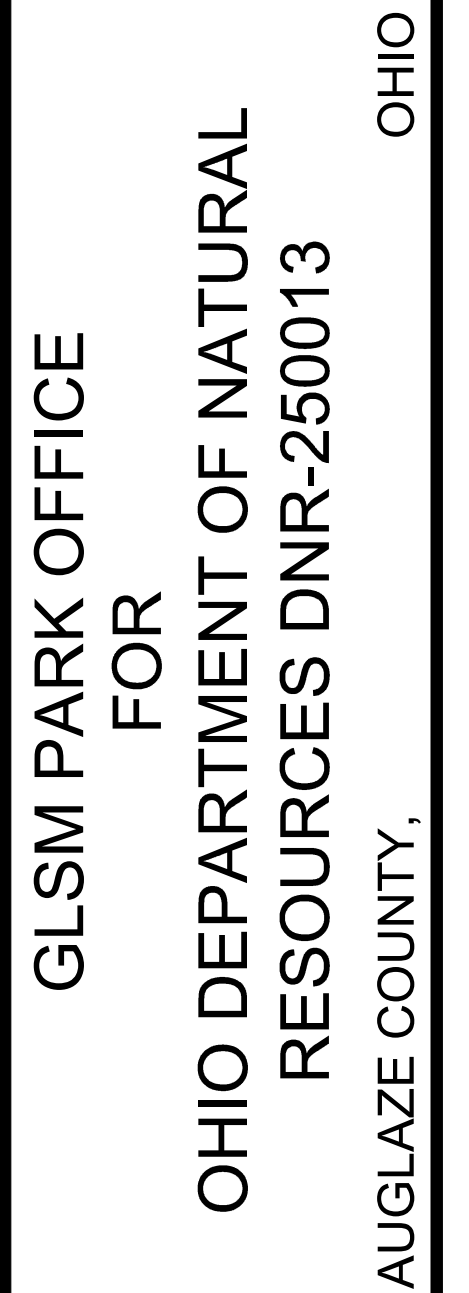
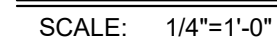
CONSTRUCTION DOCUMENTS

E300.1



THE FINAL ARRANGEMENT OF THE TECH RACK IS SUBJECT TO CHANGE BASED ON SUBMITTED EQUIPMENT SPECIFICATIONS AND OWNER-PROVIDED DEVICES. COORDINATE WITH THE OWNER'S IT TEAM PRIOR TO INSTALLATION TO ENSURE ALIGNMENT WITH CURRENT REQUIREMENTS.

THE QUANTITIES INDICATED ARE APPROXIMATE AND PROVIDED SOLELY FOR GENERAL REFERENCE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY AND FURNISH THE ACCURATE QUANTITIES AND APPROPRIATE TYPES OF CONNECTIONS REQUIRED FOR THE SCOPE OF WORK.



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CONSTRUCTION DOCUMENTS

E300.2